



Note: One- or two-way STOP-Controlled unsignalized intersections do not have an overall intersection LOS value, since the mainline through move operates freely through the intersection. Red shaded circles denote intersections/approaches operating at LOS E or F. [\*] = Unsignalized intersection requires attention due to failing minor approach movements.

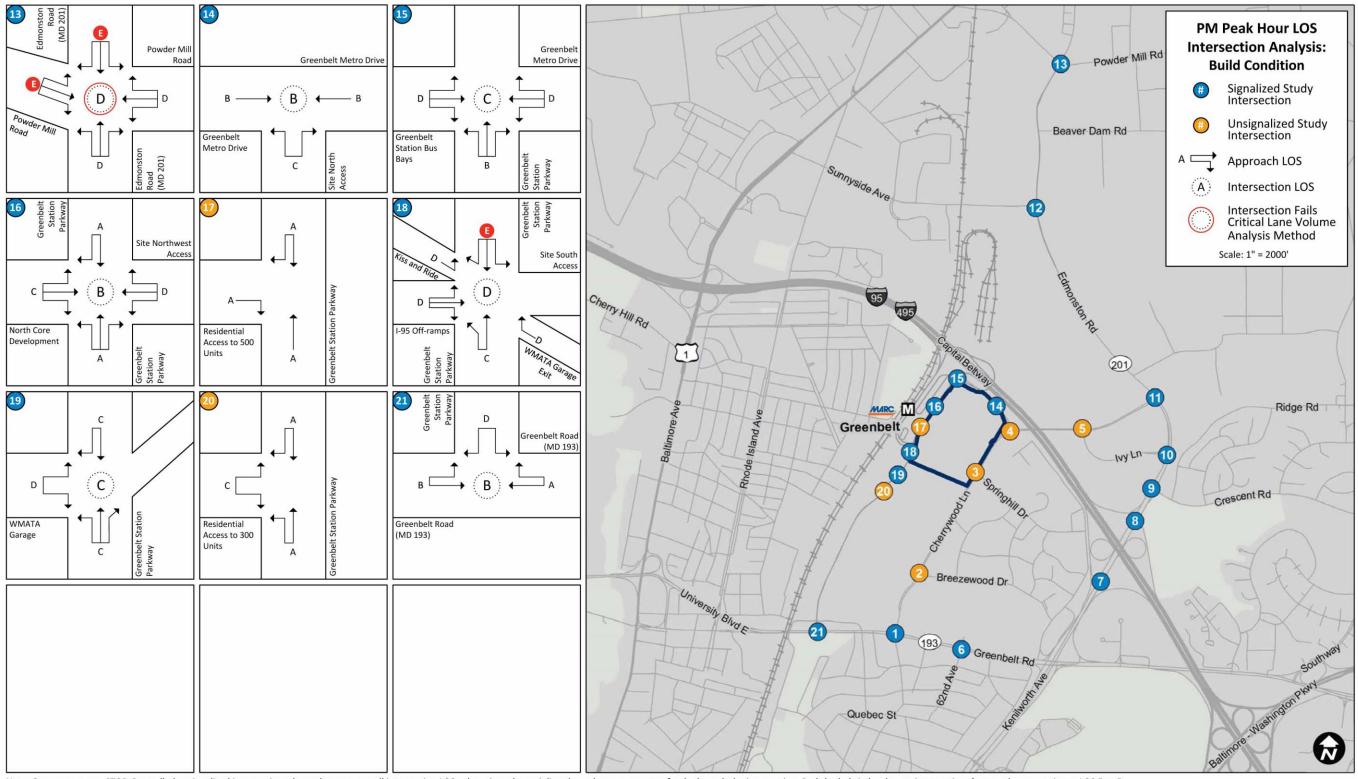


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#### Table 5-20: Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis

						No	-build	Condi	tion			-			-	В	uild C	onditio	n			
				AM	Peak l	Hour			PM	Peak H	lour			AM	Peak H	lour			РМ	Peak H	lour	
#	Intersection and Approach	Lane Group	нсм	2000	CL		Check		2000	CL	v	Check	НСМ	2000	CL	v	Check	HCM 2	2000	CL	v	Check
			Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	One or	Delay	LOS	Critical Lane Vol	LOS		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	One ex	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Oneek
1	Greenbelt Road (MD 193) & (	Cherryw	ood La	ne/6	0th Ave	nue (S	Signal	ized)										-				
	EB (Greenbelt Rd)	L	63.2	Е				53.0	D				63.1	Е				53.0	D			
	EB (Greenbelt Rd)	TR	8.8	Α				13.9	В				8.8	Α	1			13.9	В			
	EB Overall (Greenbelt Rd)		19.1	В				21.2	С				19.0	В				21.2	С			
	WB (Greenbelt Rd)	L	64.2	Е				67.0	E				64.5	Е				67.1	Е			
	WB (Greenbelt Rd)	TR	20.6	С				35.7	D				21.2	С				35.8	D			
	WB Overall (Greenbelt Rd)		21.5	С				36.9	D				22.1	С				37.0	D			
	NB (60th Ave)	LTR	74.0	Е				132.4	F				74.0	Е				132.4	F			
	NB Overall (60th Ave)		74.0	E				132.4	F				74.0	Е				132.4	F			
	SB (Cherrywood Ln)	L	76.7	Е				106.8	F				78.1	Е				138.9	F			
	SB (Cherrywood Ln)	LT	76.7	Е				108.0	F				78.2	Е				137.8	F			
	SB (Cherrywood Ln)	R	70.0	Е				83.5	F				71.2	Е				108.0	F			
	SB Overall (Cherrywood Ln)		71.9	Е				91.0	F				73.2	Е				117.8	F			
	Overall		28.5	С	1,315	D	Pass	42.2	D	1,504	E	Pass	28.9	С	1,335	D	Pass	48.3	D	1,552	E	Pass
2	Cherrywood Lane & Breezev	wood Dr	ive (AV	NSC)	=					=		-			•			-	-			
	WB (Breezewood Dr)	LR	13.3	-				12.5	-				13.4	-				13.0	-			
	WB Overall (Breezewood Dr	·)	13.3	В				12.5	В				13.4	В				13.0	В			
	NB (Cherrywood Ln)	Т	11.2	-				12.4	-				11.3	-				12.9	-			
	NB (Cherrywood Ln)	R	8.7	-				9.4	-	-			8.7	-				9.7	-			
	NB Overall (Cherrywood Ln)		10.1	В				11.1	В	-			10.2	В				11.5	В			
	SB (Cherrywood Ln)	L	9.7	-				10.5	-	-			9.7	-	1			10.6	-			
	SB (Cherrywood Ln)	Т	10.8	-				15.1	-	-			11.0	-				21.9	-			
	SB Overall (Cherrywood Ln)		10.4	в				13.7	в	-			10.6	В				19.0	С			
	Overall		11.2	В	N/A	N/A	Pass		-	N/A	N/A	Pass			N/A	N/A	Pass	15.2	C	N/A	N/A	Pass
3	Cherrywood Lane & Springh	nill Drive																	•	1471		
H	WB (Springhill Dr)	LR	16.4	с, С				128.6	F				16.5	С				176.5	F.			
	WB (Springhill Dr) WB Overall (Springhill Dr)		<b>16.4</b>	<b>c</b>	1			128.6					<b>16.5</b>	<b>c</b>				176.5	F			
	SB (Cherrywood Ln)	L	8.3		1			8.7					8.3	-				8.7	Δ			
	, ,		-	A	-			-	A					A					A			
	SB Overall (Cherrywood Ln)		3.0	-	N// A	<b>N</b> 1/A	D	2.4	-	N// A		D	3.0	-	N1/A	<b>NI</b> /A	D	2.1	-	N1/A		
	Overall		5.2	-	N/A	N/A	Pass	27.0	-	N/A	N/A	Pass	5.2	-	N/A	N/A	Pass	34.3	-	N/A	N/A	Pass

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

	ble 5-20. Comparison of NC							Condit				•			<b>,</b> (		uild Co	onditio	on			
				AM	Peak H					Peak H	lour			AM	Peak H					Peak F	lour	
#	Intersection and Approach	Lane Group	HCM		CLV	/	Check	HCM		CL\	,	Check	HCM 2		CLV	/	Check	HCM		CL		Check
			Delay (sec/ veh)		Vol	LOS	CHECK	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	CIIECK	Delay (sec/ veh)	LOS	Critical Lane Vol		CHECK	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	CHECK
4	Cherrywood Lane & Greenbe	It Metro		(Rou	ndabou	it) <sup>a</sup>																
	EB (Greenbelt Metro Dr)	LR	6.1	А				14.6	В				6.2	Α				16.5	С			
	EB Overall (Greenbelt Metro	Dr)	3.3	Α				7.5	Α				3.3	Α				7.9	Α			
	NB (Cherrywood Ln)	LT	11.8	В				14.4	В				11.8	В				15.6	С			
	NB Overall (Cherrywood Ln)		11.8	В				14.4	В				11.8	В				15.6	С			
	SB (Cherrywood Ln)	Т	6.3	А				12.0	В				6.3	Α				12.0	В			
	SB Overall (Cherrywood Ln)		2.2	Α				8.9	Α				2.0	Α				8.9	Α			
	Overall		6.0	Α	N/A	N/A	Pass	9.8	Α	N/A	N/A	Pass	5.8	Α	N/A	N/A	Pass	10.0	В	N/A	N/A	Pass
5	Cherrywood Lane & Ivy Lane	(TWSC	)		_						_				=							
	EB (Cherrywood Ln)	LTR	3.0	А				0.4	А				3.1	А				0.4	А			
	EB Overall (Cherrywood Ln)		3.0	-				0.4	-				3.1	-				0.4	-			
	WB (Cherrywood Ln)	L	8.3	А				8.8	А	1			8.3	А				8.9	А			
	WB (Cherrywood Ln)	TR	0.0	-				0.0	-	1			0.0	-				0.0	-			
	WB Overall (Cherrywood Ln)		0.4	-				0.2	-	1			0.4	-				0.2	-			
	NB (lvy Ln)	LT	67.2	F				^	F				79.7	F				^	F			
	NB (lvy Ln)	R	10.3	В				12.1	В	1			10.3	В				12.5	В			
	NB Overall (Ivy Ln)		55.7	F				^	F				65.7	F				^	F			
	SB (Ivy Ln)	LTR	41.0	Е				402.7	F				44.7	Е				443.6	F			
	SB Overall (Ivy Ln)		41.0	Е				402.7	F				44.7	Е				443.6	F			
	Overall		6.0	-	N/A	N/A	Pass	b	-	N/A	N/A	Fail	6.6	-	N/A	N/A	Pass	b	-	N/A	N/A	Fail
6	Greenbelt Road (MD 193) & 62	2 Avenu	e/Belt	way I	Plaza D	rivew	ay (Si	gnaliz	ed)													
	EB (Greenbelt Rd)	L	1.7	Α				7.0	A				1.8	А				7.1	Α			
	EB (Greenbelt Rd)	TR	2.6	А				11.3	В	1			2.7	А				12.2	В			
	EB Overall (Greenbelt Rd)		2.6	Α				11.2	В	1			2.6	Α				12.1	В			
	WB (Greenbelt Rd)	L	4.0	Α				24.7	С				4.1	А				25.4	С			
	WB (Greenbelt Rd)	Т	7.5	Α				18.3	В				7.6	А				18.3	В			
	WB (Greenbelt Rd)	R	4.7	А				14.8	В				4.7	А				14.8	В			
	WB Overall (Greenbelt Rd)		7.2	Α				17.8	В	1			7.4	Α				17.8	в			
	NB (62nd Ave)	LTR	68.1	Е				71.4	Е	1			68.1	E				71.4	Е			
	NB Overall (62nd Ave)		68.1	Е				71.4	Е	1			68.1	Е				71.4	Е			
	SB (Beltway Plaza Drwy)	L	68.2	Е				69.8	Е	1			68.2	Е				69.8	Е			
	SB (Beltway Plaza Drwy)	LT	68.3	Е				69.5	Е	1			68.3	Е				69.5	Е			
	SB (Beltway Plaza Drwy)	R	66.7	Е				54.9	D	1			66.7	Е				54.9	D			
	SB Overall (Beltway Plaza Dr		67.8	Е				67.1	Е	1			67.8	Е				67.1	Е			
	Overall	.,	7.5	Α	742	Α	Pass	20.4	С	1,206	С	Pass	7.6	Α	757	Α	Pass	20.7	С	1,220	С	Pass

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hou	r Operations Analysis (continued)
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					No	-build	Condit	tion							В	uild C	onditic	on			
			AM	Peak H	lour			PM	Peak F	lour			AM	Peak H	lour			PM	Peak H	lour	
# Intersection and Approach	Lane Group	HCM	2000	CL	V		НСМ	2000	CL	/	Ohaala	HCM :	2000	CL\			HCM	2000	CL	V	01
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check
7 Kenilworth Avenue (MD 201)	) & I-95/I-	495 SE	Off-r	ramp (S	igna	ized)					_										
EB (I-95/I-495 SB Off-ramp)	L	39.7	D				39.7	D				39.7	D				39.7	D			
EB (I-95/I-495 SB Off-ramp)	R	6.9	А				0.6	А				7.0	А				0.6	Α			
EB Overall (I-95/I-495 SB Off	-ramp)	13.8	В				14.9	В				13.9	в				14.7	В			
NB (Kenilworth Ave)	Т	4.0	Α				4.0	Α				4.0	А				4.0	Α			
NB Overall (Kenilworth Ave)	)	4.0	Α				4.0	Α				4.0	Α				4.0	Α			
SB (Kenilworth Ave)	Т	6.2	Α				3.6	Α				6.2	А				3.6	Α			
SB Overall (Kenilworth Ave)	)	6.2	Α				3.6	Α				6.2	Α				3.6	Α			
Overall		9.1	Α	730	Α	Pass	6.8	Α	593	Α	Pass	9.1	Α	730	Α	Pass	6.8	Α	594	Α	Pass
8 Kenilworth Avenue (MD 201	) & I-95/I-	495 NB	G Off-r	ramp (S	igna	ized)														·	
WB (I-95/I-495 NB Off-ramp)	L	24.6	С		<u> </u>	,	34.3	С				24.5	С				34.3	С			
WB (I-95/I-495 NB Off-ramp)	R	26.3	С	1			31.1	С	-			26.2	С				31.1	С			
WB Overall (I-95/I-495 NB Of	f-ramp)	25.4	С				32.8	С				25.3	С				32.8	С			
NB (Kenilworth Ave)	Т	11.1	В				5.4	Α				11.2	В				5.4	Α			
NB Overall (Kenilworth Ave)	)	11.1	В				5.4	Α				11.2	в				5.4	Α			
SB (Kenilworth Ave)	Т	7.7	Α				3.4	Α				7.8	А				3.3	Α			
SB Overall (Kenilworth Ave)	)	7.7	Α	1			3.4	Α	-			7.8	Α				3.3	Α			
Overall	·	16.7	В	868	Α	Pass	13.3	В	779	Α	Pass	16.7	В	868	Α	Pass	13.3	В	781	Α	Pass
9 Kenilworth Avenue (MD 201	) & Cresc	ent Ro	ad/M	aryland		Office	e (Sign	alize	d)												
EB (Maryland SHA Office)	LTR	26.0	С				36.1	D	Ĺ			26.0	С				36.1	D			
EB Overall (Maryland SHA C	) Dffice)	26.0	С				36.1	D				26.0	С				36.1	D			
WB (Crescent Rd)	, LT	43.2	D				47.8	D				43.2	D				47.8	D			
WB (Crescent Rd)	R	26.6	С				36.3	D				26.6	С				36.3	D			
WB Overall (Crescent Rd)	ļ	38.0	D				43.0	D				38.0	D				43.0	D			
NB (Kenilworth Ave)	L	47.4	D				61.5	Е				47.3	D				61.5	Е			
NB (Kenilworth Ave)	Т	13.3	В				10.4	В				13.4	В				10.4	В			
NB (Kenilworth Ave)	R	8.5	A	1			5.9	A				8.6	A				5.9	A			
NB Overall (Kenilworth Ave)		13.9	B	1			10.2	B				14.0	В				10.2	B			
SB (Kenilworth Ave)	L	67.1	Е	1			53.3	D	1			67.0	Е				53.8	D			
SB (Kenilworth Ave)	Т	4.7	А	1			5.8	A				4.7	A				5.8	A			
SB (Kenilworth Ave)	R	12.0	B	1			4.9	A				12.0	B				4.9	A			
SB Overall (Kenilworth Ave)		9.3	A	1			11.1	B				9.3	A				11.2	B			
Overall	,	15.1	B	962	Α	Pass		В	796	Α	Pass		В	965	Α	Pase	12.9	В	798	Α	Pass

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

					No	-build	Condit	ion			-				В	uild Co	onditio	n			
			AM	Peak H					Peak H	lour			AM	Peak H					Peak F	lour	
# Intersection and Approach	Lane Group	нсм	2000	CLV	V	Chask	НСМ	2000	CLV		Check	HCM 2	2000	CL\	/	Chask	HCM	2000	CL	v	Chash
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)		Critical Lane Vol		Check
10 Kenilworth Avenue (MD 201	) & Ivy La	ane (Si	gnali	zed)																	
EB (lvy Ln)	R	0.1	Α				0.7	А				0.1	А				0.8	А			
EB Overall (Ivy Ln)		0.1	Α				0.7	Α				0.1	Α				0.8	Α			
NB (Kenilworth Ave)	L	18.6	В				25.8	С				18.6	В				25.8	С			
NB (Kenilworth Ave)	Т	0.3	Α				0.2	Α	1			0.3	Α	1			0.2	А			
NB Overall (Kenilworth Ave	)	3.4	Α				1.7	Α				3.4	Α				1.7	Α			
SB (Kenilworth Ave)	Т	0.7	Α				1.2	Α				0.7	А				1.2	А			
SB (Kenilworth Ave)	R	0.0	Α				0.0	Α				0.0	А				0.0	А			
SB Overall (Kenilworth Ave	)	0.7	Α				1.2	Α				0.7	Α				1.2	Α			
Overall		2.3	Α	784	Α	Pass	1.3	Α	761	Α	Pass	2.3	Α	784	Α	Pass	1.3	Α	761	Α	Pass
11 Kenilworth Avenue/Edmons	ton Road	i (MD 2	201) &	Cherry	wood	d Lane	(Sign	alized	d)							-	_				
EB (Cherrywood Ln)	L	46.7	D				39.4	D				46.8	D				39.8	D			
EB (Cherrywood Ln)	R	40.7	D				33.8	С	1			40.7	D				33.3	С			
EB Overall (Cherrywood Ln)	)	45.7	D				37.5	D				45.8	D				37.6	D			
NB (Kenilworth Ave)	L	27.0	С				13.8	В				27.6	С				14.0	В			
NB (Kenilworth Ave)	Т	1.1	Α				1.2	Α				1.1	А				1.3	А			
NB Overall (Kenilworth Ave	)	11.1	В				3.5	Α				11.5	В				3.6	Α			
SB (Edmonston Rd)	Т	22.6	С				13.9	В				23.0	С				14.3	В			
SB (Edmonston Rd)	R	17.5	В				10.0	В				18.5	В				10.3	В			
SB Overall (Edmonston Rd)	•	21.2	С				13.2	В				21.7	С				13.6	В			
Overall		18.8	В	1,212	С	Pass	14.7	В	990	Α	Pass	19.2	В	1,221	С	Pass	15.2	В	1,008	В	Pass
12 Edmonston Road (MD 201) 8	k Sunnysi	ide Av	enue	(Signal	ized)											_					
EB (Sunnyside Ave)	L	108.9	F				113.0	F				122.5	F				113.0	F			
EB (Sunnyside Ave)	R	66.9	Е				62.0	Е				72.7	Е				62.0	Е			
EB Overall (Sunnyside Ave)	•	77.9	Е				80.1	F				85.8	F				80.1	F			
NB (Edmonston Rd)	L	102.8	F				98.0	F				117.6	F				98.0	F			
NB (Edmonston Rd)	Т	4.4	Α				18.3	В	1			4.2	А				20.3	С			
NB Overall (Edmonston Rd)		29.6	С				33.3	С				33.2	С				34.6	С			
SB (Edmonston Rd)	Т	41.1	D				48.1	D				43.3	D				48.1	D			
SB (Edmonston Rd)	R	5.0	Α				3.8	Α				5.0	А				3.8	Α			
SB Overall (Edmonston Rd)		35.6	D				41.4	D				37.6	D				41.4	D			
Overall		40.1	D	1,486	Е	Pass	46.7	D	1,692	F	Fail	43.6	D	1,516	Е	Pass	47.1	D	1,722	F	Fail

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

					No	-build	Condi	tion							E	Build Co	onditio	on			
			AM	Peak H	lour			PM	Peak l	Hour			AM	Peak H	lour			PM	Peak H	lour	
# Intersection and Approach	Lane Group	НСМ	2000	CL	V		нсм	2000	CL	v	Ohaada	HCM 2	2000	CLV	V		нсм	2000	CL	v	0
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check
13 Edmonston Road (MD 201) 8	& Powder	Mill R	load (	Signali	zed)																
EB (Powder Mill Rd)	L	47.3	D				45.2	D				47.6	D				45.2	D			
EB (Powder Mill Rd)	Т	62.8	Е				81.1	F				63.4	Е				81.1	F			
EB (Powder Mill Rd)	R	48.7	D				44.7	D	1			49.2	D	1			44.7	D			
EB Overall (Powder Mill Rd	)	52.8	D				60.5	Е				53.3	D				60.5	E			
WB (Powder Mill Rd)	L	57.0	Е				84.1	F				58.4	Е				84.1	F			
WB (Powder Mill Rd)	Т	41.8	D				38.4	D	1			42.2	D				38.4	D			
WB (Powder Mill Rd)	R	35.6	D				34.1	С				35.9	D				34.1	С			
WB Overall (Powder Mill Ro	) (k	46.9	D				53.4	D	1			47.7	D	1			53.4	D			
NB (Edmonston Rd)	L	48.5	D				76.7	Е				49.8	D	1			85.4	F			
NB (Edmonston Rd)	Т	12.8	В	1			23.2	С	1			12.7	В	1			23.6	С			
NB (Edmonston Rd)	R	8.4	Α				12.5	В	1			8.3	Α	1			12.5	В			
NB Overall (Edmonston Rd)		29.7	С				41.3	D	1			30.3	С	1			45.1	D			
SB (Edmonston Rd)	L	40.5	D				54.5	D				40.6	D				54.7	D			
SB (Edmonston Rd)	TR	52.5	D				60.4	Е				53.1	D				60.4	Е			
SB Overall (Edmonston Rd)		52.0	D				59.8	Е				52.7	D	1			59.8	Е			
Overall		42.5	D	1,593	Е	Pass	50.9	D	1,867	F	Fail	43.2	D	1,595	Е	Pass	52.6	D	1,897	F	Fail
14 Greenbelt Metro Drive & Sit	e North A	Access	(Sigr	alized)	b																
EB (Greenbelt Metro Dr)	Т	N/A	N/A				N/A	N/A				3.5	А				17.5	В			
EB Overall (Greenbelt Metro	o Dr)	N/A	N/A				N/A	N/A				3.5	Α				17.5	В			
WB (Greenbelt Metro Dr)	L	N/A	N/A				N/A	N/A				-	-				-	-			
WB (Greenbelt Metro Dr)	Т	N/A	N/A				N/A	N/A				4.4	Α				11.5	В			
WB Overall (Greenbelt Metr	ro Dr)	N/A	N/A				N/A	N/A				4.4	Α				11.5	В			
NB (Site North Access)	L	N/A	N/A				N/A	N/A				22.7	С				25.9	С			
NB (Site North Access)	R	N/A	N/A				N/A	N/A				21.4	С				15.9	В			
NB Overall (Site North Acce	ss)	N/A	N/A				N/A	N/A				22.4	С		-		23.6	C			
Overall		N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	4.9	Α	605	Α	Pass	18.2	В	1,029	В	Pass

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

						No-	build	Condit	ion							В	uild C	onditic	on			
				AM	Peak H	lour			PM	Peak H	lour			AM	Peak H	lour			PM	Peak H	lour	
#	Intersection and Approach	Lane Group	HCM	2000	CL		Check	НСМ	2000	CL	v	Check	НСМ	2000	CL	v	Check	HCM	2000	CL	v	Check
			Delay (sec/ veh)	LOS	Critical Lane Vol		CHECK	Delay (sec/ veh)	LOS	Critical Lane Vol		GIIECK	Delay (sec/ veh)	LOS	Critical Lane Vol		Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	
15	Greenbelt Station Bus Bays/	Greenbe	lt Metr	o Dri	ve & G	reenb	elt Sta	tion P	arkw	ay (Sig	nalize	ed)										
	EB (Greenbelt Sta Bus Bays)	LT	75.7	ш				54.0	D				75.7	ш				54.0	D			
	EB (Greenbelt Sta Bus Bays)	R	-	-				-	-				-	-				-	-			
	EB Overall (Greenbelt Sta B	us Bays)	75.7	Е				54.0	D				75.7	E				54.0	D			
	WB (Greenbelt Metro Dr)	L	56.6	Е				45.2	D	1			54.4	D	1			40.9	D			
	WB (Greenbelt Metro Dr)	Т	35.7	D	1			31.7	С	1			33.1	С	1			30.5	С			
	WB (Greenbelt Metro Dr)	R	36.0	D				31.5	С				33.8	С				36.5	D			
	WB Overall (Greenbelt Metro	o Dr)	52.1	D				41.6	D				49.4	D				37.8	D			
	NB (Greenbelt Sta Pkwy)	L	-	-	1			-	-	1			-	-	1			-	-			
	NB (Greenbelt Sta Pkwy)	Т	14.3	В	1			8.4	Α	1			16.2	В	1			11.1	В			
	NB (Greenbelt Sta Pkwy)	R	13.8	В				21.4	С	1			24.2	С				18.6	В			
	NB Overall (Greenbelt Sta P	kwy)	14.0	В				16.2	В	1			19.9	В				15.8	В			
	Overall		31.4	С	644	Α	Pass	23.3	С	603	Α	Pass	34.3	С	682	Α	Pass	25.2	С	813	Α	Pass
16	Greenbelt Station Parkway 8	& North C	Core D	evelo	pment	/Site N	orthw	est Ac	cess	(Signal	ized)											
	EB (North Core Dev)	L	69.2	Е				42.1	D				69.5	E				30.2	С			,
	EB (North Core Dev)	TR	66.5	Е				35.0	С	1			65.7	Е				26.0	С			
	EB Overall (North Core Dev)		68.8	Е				40.7	D				68.8	E				29.4	С			
	WB (Site Northwest Access)	LTR (AM)	-	-	1			-	-				63.0	Е				-	-			
	WB (Site Northwest Access)	LT (PM)	-	-				-	-	1			-	-	1			-	-			
	WB (Site Northwest Access)	R (PM)	-	-	1			-	-	1			-	-	1			54.7	D			
	WB Overall (Site Northwest	Access)	-	-				-	-	1			63.0	Е				54.7	D			
	NB (Greenbelt Sta Pkwy)	L	3.9	Α				3.6	Α	1			3.0	Α	1			6.9	Α			
	NB (Greenbelt Sta Pkwy)	TR	2.2	Α				3.4	Α	1			2.9	Α				6.7	Α			
	NB Overall (Greenbelt Sta P	kwy)	2.7	Α	1			3.4	Α	1			2.9	Α	1			6.7	Α			
	SB (Greenbelt Sta Pkwy)	TR	0.1	Α				0.1	Α				0.1	Α				0.1	Α			
	SB Overall (Greenbelt Sta P	kwy)	0.1	Α	1			0.1	Α	1			0.1	Α	1			0.1	Α			
	Overall		5.4	Α	600	Α	Pass		В	460	Α	Pass	5.7	Α	976	Α	Pass		В	952	Α	Pass
17	Greenbelt Station Parkway	& Reside	ntial A	cces	s to 500	Units	(TWS	C)			•											
	EB (Residential Access)	R	9.8	Α			•	9.3	Α				10.0	Α				9.3	Α			
	EB Overall (Residential Acce	ess)	9.8	Α	1			9.3	Α	1			10.0	Α	1			9.3	Α			
	Overall	-	0.6	-	N/A	N/A	Pass	0.2	-	N/A	N/A	Pass	0.5	-	N/A	N/A	Pass	0.2	-	N/A	N/A	Pass

Table 5-20:	Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

			No-build Condition Build C								ondition											
				AM	Peak H	lour			РМ	Peak H	our			AM	Peak H	lour			PM	Peak H	lour	
#	Intersection and Annroach	Lane Group	HCM :	2000	CL		Check	НСМ	2000	CLV	,	Check	HCM 2	2000	CL	v	Check	HCM	2000	CL		Check
			Delay (sec/ veh)		Critical Lane Vol		Chicok	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS		Delay (sec/ veh)	LOS	Critical Lane Vol		Chicox
18	Greenbelt Station Parkway & I	-95/1-49	95 Off-	ramp	s/Site S	South	Acces	s/Kiss	& Rid	le (Sign	alize	d)										
	EB (I-95 Off-ramps)	L	71.7	E				44.8	D				282.5	F				46.0	D			
	EB (I-95 Off-ramps)	LTR	56.3	E				31.2	С				204.4	F				31.2	С			
	EB Overall (I-95 Off-Ramps)		61.7	E				36.1	D				230.3	F				36.5	D			
	EB (Kiss and Ride)	L	55.9	E				37.5	D				57.9	E				38.2	D			
	EB Overall (Kiss and Ride)		55.9	E				37.5	D				57.9	E				38.2	D			
	WB (Site South Access)	R	37.0	D				35.7	D				42.2	D				35.7	D			
	WB Overall (Site South Access	5)	37.0	D				35.7	D				42.2	D				35.7	D			
	NB (Greenbelt Sta Pkwy)	L	81.8	F				33.4	С				79.5	Е				33.5	С			
	NB (Greenbelt Sta Pkwy)	Т	30.9	С				23.5	С				39.2	D				23.7	С			
	NB Overall (Greenbelt Sta Pkw	/y)	32.7	С				24.3	С				40.5	D				24.4	С			
	SB (Greenbelt Sta Pkwy)	L	2.9	А				84.5	F				51.5	D				84.5	Т			
	SB (Greenbelt Sta Pkwy)	TR	6.6	А				76.4	Е				10.1	В				76.3	Ш			
	SB Overall (Greenbelt Sta Pkw	/y)	5.7	Α				77.7	E				22.7	С				77.5	ш			
	Overall		40.0	D	950	Α	Pass	36.9	D	1,103	В	Pass	141.0	F	1,514	Е	Fail	37.1	D	1,129	В	Pass
19	Greenbelt Station Parkway & W	WMAT	A Gara	ge (S	Signaliz	zed)																
	EB (WMATA Garage)	L	76.3	E				51.0	D				76.3	Е				51.0	D			
	EB (WMATA Garage)	R	72.4	E				37.8	D				72.4	Е				37.8	D			
	EB Overall (WMATA Garage)		74.9	E				49.3	D				74.9	Ш				49.3	D			
	NB (Greenbelt Sta Pkwy)	LT	65.7	Е				51.5	D				63.4	Ш				51.4	D			
	NB (Greenbelt Sta Pkwy)	TR	3.0	А				4.6	Α				4.1	А				4.6	А			
	NB Overall (Greenbelt Sta Pkw	/y)	34.7	С				28.3	С				33.8	С				28.2	С			
	SB (Greenbelt Sta Pkwy)	Т	18.8	В				20.5	С				21.0	С				20.5	С			
	SB (Greenbelt Sta Pkwy)	R	38.5	D				12.9	В	1			41.2	D				12.9	В			
	SB Overall (Greenbelt Sta Pkw	vy)	25.5	С				20.3	С	1			27.9	С				20.3	С			
	Overall		31.4	С	429	Α	Pass	27.8	С	524	Α	Pass	32.0	С	480	Α	Pass	27.8	С	524	Α	Pass
20	Greenbelt Station Parkway & F	Reside	ntial A	cces	s to 300	) Units	(TWS	C)		_												
	EB (Residential Access)	LR	21.1	С				20.8	С				24.4	С				20.9	С			
	EB Overall (Residential Access	5)	21.1	С				20.8	С	1			24.4	С				20.9	С			
	NB (Greenbelt Sta Pkwy)	ĹT	0.2	Α				0.8	Α	1			0.2	Α				0.7	Α			
	NB Overall (Greenbelt Sta Pkw	/y)	0.1	-				0.3	-	1			0.1	-				0.3	-			
	Overall		1.5	-	N/A	N/A	Pass	0.6	-	N/A	N/A	Pass	1.6	-	N/A	N/A	Pass	0.6	-	N/A	N/A	Pass

#### Table 5-20: Comparison of No-build and Build Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

			No-build Condition											E	Build C	onditio	on					
				AM	Peak H	lour			PM	Peak H	lour			AM	Peak H	lour			PM	Peak H	lour	
#	Intersection and Approach	Lane Group	НСМ	2000	CL		Check	НСМ	2000	CL		Check	HCM 2000	CL		Check	НСМ	2000	CLV	V	Check	
			Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	S	Delay	LOS	Critical Lane Vol	LOS	Check	Delay	LOS	Critical Lane Vol	LOS	Check	Delay	LOS	Critical Lane Vol	LOS	
21	Greenbelt Road (MD 193) &	Greenbe	lt Stati	ion Pa	arkway	(Sigr	nalized	)		_					_		-	-				
	EB (Greenbelt Rd)	L	63.6	E				70.0	Е				67.5	E				70.0	E			
	EB (Greenbelt Rd)	Т	3.2	Α				8.0	Α				3.3	Α				8.0	Α			
	EB Overall (Greenbelt Rd)		11.5	В				12.6	В				14.0	В				12.7	В			
	WB (Greenbelt Rd)	Т	3.6	Α				4.9	Α				3.5	Α				5.1	Α			
	WB (Greenbelt Rd)	R	0.1	Α				1.8	Α				0.3	Α				1.9	Α			
	WB Overall (Greenbelt Rd)		3.2	Α				4.5	Α				3.2	Α				4.6	Α			
	SB (Greenbelt Sta Pkwy)	L	67.1	Е				59.9	Е				65.8	Е				59.8	Е			
	SB (Greenbelt Sta Pkwy)	R	46.0	D				47.4	D				44.6	D				47.3	D			
	SB Overall (Greenbelt Sta P	kwy)	57.5	E				54.1	D				56.2	E				54.0	D			
	Overall		11.1	В	988	Α	Pass	12.7	В	1,100	В	Pass	11.7	В	1,020	В	Pass	12.7	В	1,101	В	Pass

Notes:

AWSC = All-way STOP-Controlled intersection

EB = Eastbound, WB = Westbound, NB= Northbound, SB = Southbound

LTR = left / through / right lanes

LOS = Level of Service

TWSC = Two-way STOP-Controlled unsignalized intersection (TWSC intersections do not have an overall LOS)

Delay is Measured in Seconds Per Vehicle.

Red cells denote intersections or approaches operating at unacceptable conditions.

^ Highway Capacity Manual was unable to report accurate delay using default gap acceptance values.

<sup>a</sup> Highway Capacity Software 2010 Roundabout results

<sup>b</sup> Intersection would be included under the Build Condition, but was included as part of the No-build Condition

design provided by Renard Development Company, LLC.

### 5.8.6 Build Condition Queuing Analysis

Synchro<sup>™</sup> was used to calculate the 50th percentile queue lengths and SimTraffic<sup>™</sup> was used to calculate the 95th percentile queue lengths. The SimTraffic simulations have a statistical accuracy of plus or minus 5.0 percent error for the AM and PM peak hour simulations.

#### 5.8.6.1 Signalized Intersection Operations Analysis

Based on the Synchro<sup>™</sup> and SimTraffic<sup>™</sup> analysis, the following signalized intersection approaches would experience failing queue lengths in Synchro<sup>™</sup> or SimTraffic<sup>™</sup> (queue exceeds available lane storage). The lane group within the approach that would be operating under unacceptable conditions is noted in parentheses

- Greenbelt Road (MD 193) and Cherrywood Lane/60th Avenue (Intersection #1)
  - o Southbound Cherrywood Lane (all movements) during the PM peak hour
- Kenilworth Avenue/Edmonston Road (MD 201) and Cherrywood Lane (Intersection #11)
  - o Southbound Edmonston Road (right turns) during the AM peak hour
- Edmonston Road (MD 201) and Sunnyside Avenue (Intersection #12)
  - Eastbound Sunnyside Avenue (right turns) and southbound Edmonston Road (right turns and through movements) during the AM peak hour
  - Eastbound Sunnyside Avenue (all movements), northbound Edmonston Road (all movements) and southbound Edmonston Road (all movements) during the PM peak hour
- Edmonston Road (MD 201) and Powder Mill Road (Intersection #13)
  - Northbound Edmonston Road (left turns) during the PM peak hour
- Greenbelt Metro Drive and Site North Access (unsignalized in No-build) (Intersection #14)
  - Eastbound Greenbelt Metro Drive (through movements) and northbound site north access (left turns) during the PM peak hour
- Greenbelt Station Bus Bays/Greenbelt Metro Drive and Greenbelt Station Boulevard (Intersection #15)
   Westbound Greenbelt Metro Drive (left turns) during the AM peak hour
- Greenbelt Station Parkway and North Core Development/Site Northwest Access (Intersection #16)
  - Westbound site northwest access (PM right turns) during the PM peak hour
- Greenbelt Station Parkway and I-95/I-495 Off-ramps/Site South Access/Kiss & Ride (Intersection #18)
  - Eastbound Kiss & Ride (left turns) and eastbound I-95 off ramps (all movements) during the AM peak hour
- Greenbelt Station Parkway and WMATA Garage (Intersection #19)
  - Eastbound WMATA garage (left turns) during the PM peak hour
- Greenbelt Road (MD 193) and Greenbelt Station Parkway (Intersection #21)
  - Eastbound Greenbelt Road (left turns), westbound Greenbelt Road (right turns) and southbound Greenbelt Station Parkway (right turns) during the PM peak hour

#### 5.8.6.2 Unsignalized Intersection Operations Analysis

Four of the six unsignalized intersections would not experience failing queue lengths for the 95th percentile. The intersection of Cherrywood Lane and Ivy Lane (Intersection #5) would experience 95th percentile failing queues on southbound Ivy Lane (combined left, through, and right movements) during the PM peak hour, and the intersection of Cherrywood Lane and Greenbelt Metro Drive would experience 95th percentile failing queues on northbound Cherrywood Lane (combined left and through movements) during the PM peak hour.

#### 5.8.6.3 Complete Intersection Queuing Analysis

This section summarizes the differences in queuing impacts between the Build Condition and the No-build Condition by quantifying the change in intersection queuing failures. Following the summary, this section also includes the complete results of the queuing analysis.

Based on the Synchro<sup>™</sup> and SimTraffic<sup>™</sup> analysis, 10 signalized and 2 unsignalized intersections would experience queuing lengths that would exceed the available storage capacity. The remaining intersections in the study area would provide sufficient storage for the anticipated demand. Compared to the No-build Condition, the Build Condition would have no change in the number of intersections with failing queues during the AM peak hour and three more intersections would have failing queues during the PM peak hour. In the AM peak hour in the No-build Condition, there would be four intersections with a failing queue approach compared with four in the Build Condition, an increase of zero. In the PM peak hour in the No-build Condition, there would be six intersections with a failing queue approach compared of 3.

Table 5-21 provides a summary of the number of intersections that meet the following criteria for approach lane groups in a queue that would change between the No-build and the Build Conditions:

Type of Change Between Conditions	AM	РМ
New Failing Movement	0	3
Additional Failing Movement	1	1
No Change	20	17
Fewer Failing Movements	0	0
No Failing Movements	0	0
Total Signalized and Unsignalized Intersections	21	21

 Table 5-21:
 Queuing Summary Comparing No-build Condition to Build Condition

The results of the No-build Condition compared to the Build Condition queuing analysis for both signalized and unsignalized intersections are presented in table 5-22. Note that the percentile values are expressed in feet, and a car occupies about 25 linear feet of roadway, including the space between cars.

					No-build	Condition			Build Co	ondition	
	Intersection and	Lane	Turning	AM F	Peak	PM F	Peak	AM F	Peak	PM F	Peak
#	Approach	Group	Bay/Link Length (feet)	50th	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)
1	Greenbelt Road (MD 193) 8	Cherryw	ood Lane	/60th Aver	nue (Signa	lized)					
	EB (Greenbelt Rd)	L	350	132	165	240	250	132	161	240	264
	EB (Greenbelt Rd)	TR	1,584	148	128	373	294	148	134	373	307
	WB (Greenbelt Rd)	L	200	43	126	68	137	43	128	69	141
	WB (Greenbelt Rd)	TR	1,336	598	324	208	296	626	336	208	301
	NB (60th Ave)	LTR	320	132	217	154	#357	132	205	154	319
	SB (Cherrywood Ln)	L	350	74	112	172	254	76	112	~204	#403
	SB (Cherrywood Ln)	LT	1,300	75	134	178	315	77	132	~210	621
	SB (Cherrywood Ln)	R	1,300	252	259	653	529	258	250	~796	693
2	Cherrywood Lane & Breez	ewood D	rive (AWS	C)							
	WB (Breezewood Dr)	LR	573	-	86	-	76	-	83	-	77
	NB (Cherrywood Ln)	Т	1,300	-	120	-	162	-	119	-	156
	NB (Cherrywood Ln)	R	1,300	-	81	-	113	-	80	-	116
	SB (Cherrywood Ln)	L	175	-	57	-	65	-	57	-	66
	SB (Cherrywood Ln)	Т	2,394	-	73	-	85	-	73	-	99
3	Cherrywood Lane & Spring	ghill Drive	e (TWSC)								
	WB (Springhill Dr)	LR	620	-	90	-	189	-	86	-	246
	NB (Cherrywood Ln)	TR	2,394	-	-	-	3	-	2	-	2
	SB (Cherrywood Ln)	L	350	-	53	-	68	-	52	-	72
4	Cherrywood Lane & Green	belt Metr	o Drive (R	oundabou	it)	_				_	
	EB (Greenbelt Metro Dr)	L	449	-	59	-	109	-	55	-	192
	EB (Greenbelt Metro Dr)	R	250	-	25	-	43	0	18	-	104
	NB (Cherrywood Ln)	LT	111	-	92	-	107	-	94	-	#118
	SB (Cherrywood Ln)	Т	1,451	-	42	-	83	0	43	-	86
	SB (Cherrywood Ln)	R	200	-	13	-	10	-	16	-	11
5	Cherrywood Lane & Ivy La		C)								
	EB (Cherrywood Ln)	LTR	1,451	-	156	-	45	-	162	-	43
	WB (Cherrywood Ln)	L	219	-	35	-	23	-	34	-	23
	WB (Cherrywood Ln)	TR	219	-	12	-	9	-	12	-	5
	NB (lvy Ln)	LT	485	-	81	-	131	_	85	-	130
	NB (lvy Ln)	R	485	-	38	-	53	-	37	-	52
	SB (lvy Ln)	LTR	223	-	66	-	#287	-	59	-	#288

#### Table 5-22: Comparison of No-build to Build Condition Queuing Analysis

					No-build	Condition	,		Build C	ondition			
			Turning	AM	Peak		Peak	AM F	Peak		Peak		
#	Intersection and	Lane	Bay/Link	50th	95th	50th	95th	50th	95th	50th	95th		
	Approach	Group	Length			Percentile							
			(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)		
6	Greenbelt Road (MD 193) 8	62 Aven	ue/Beltwa	y Plaza D	riveway (S	ignalized)	)						
	EB (Greenbelt Rd)	L	250	0	27	9	63	0	25	10	68		
	EB (Greenbelt Rd)	TR	1,336	63	56	511	221	64	49	526	229		
	WB (Greenbelt Rd)	L	250	9	53	19	123	9	52	19	106		
	WB (Greenbelt Rd)	Т	1,038	190	168	373	291	197	167	375	287		
	WB (Greenbelt Rd)	R	1,038	0	39	3	96	0	36	3	103		
	NB (62th Ave)	LTR	697	25	96	115	202	25	93	115	205		
	SB (Beltway Plaza Drwy)	L	350	16	14	173	238	16	7	173	251		
	SB (Beltway Plaza Drwy)	LT	472	17	69	172	268	17	65	172	272		
	SB (Beltway Plaza Drwy)	R	350	0	23	0	51	0	22	0	50		
7	Kenilworth Avenue (MD 20	1) & I-95/I	-495 SB O	ff-ramp (S	ignalized)								
	EB (I-95/I-495 SB Off-ramp)	L	531	112	300	97	211	112	303	97	209		
	EB (I-95/I-495 SB Off-ramp)	R	736	0	394	0	2	0	337	0	-		
	NB (Kenilworth Ave)	Т	1,263	46	90	66	116	47	89	66	117		
	SB (Kenilworth Ave)	Т	574	229	180	56	115	229	179	57	118		
8	Kenilworth Avenue (MD 20	1) & I-95/I	-495 NB O	ff-ramp (S	ignalized)								
	WB (I-95/I-495 NB Off-ramp)	L	885	223	245	160	222	223	244	160	228		
	WB (I-95/I-495 NB Off-ramp)	R	835	217	152	61	96	217	155	61	89		
	NB (Kenilworth Ave)	Т	345	116	131	49	94	119	132	49	101		
	SB (Kenilworth Ave)	Т	199	56	154	77	129	56	156	78	131		
9	Kenilworth Avenue (MD 20	-		/Maryland	SHA Offic	e (Signali	zed)						
	EB (Maryland SHA Office)	LTR	250	1	36	3	48	1	38	3	48		
	WB (Crescent Rd)	LT	441	168	254	79	145	168	263	79	149		
	WB (Crescent Rd)	R	250	0	133	0	71	0	135	0	78		
	NB (Kenilworth Ave)	L	250	28	85	9	36	28	86	9	40		
	NB (Kenilworth Ave)	Т	286	234	281	117	160	234	282	117	162		
	NB (Kenilworth Ave)	R	250	9	114	2	35	9	122	2	43		
	SB (Kenilworth Ave)	L	300	64	110	128	201	64	110	131	201		
	SB (Kenilworth Ave)	Т	793	45	156	60	446	45	155	60	452		
	SB (Kenilworth Ave)	R	R	0	10	0	194	0	11	0	185		
10	Kenilworth Avenue (MD 20	1) & Ivy L	ane (Sign	alized)									
	EB (lvy Ln)	R	-	0	-	0	-	0	-	0	-		
	NB (Kenilworth Ave)	L	547	88	134	21	59	87	136	21	59		
	NB (Kenilworth Ave)	Т	-	45	64	29	-	45	110	29	-		
	SB (Kenilworth Ave)	Т	1,198	4	93	15	101	4	96	16	108		
	SB (Kenilworth Ave)	R	-	0	-	0	-	0	-	0	-		

#### Table 5-22: Comparison of No-build to Build Condition Queuing Analysis (continued)

					No-build	Condition	-		Build Co	ondition	
			Turning	AM I	Peak	PM F	Peak	AM F	Peak	PM F	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length	50th	95th	50th	95th	50th	95th	50th	95th
	, pprodon	Creap	(feet)	Percentile	Percentile	Percentile		Percentile	Percentile	Percentile	Percentile
				(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
11	Kenilworth Avenue/Edmor	ston Roa	d (MD 201)	) & Cherry	wood Lan	e (Signaliz	zed)				
	EB (Cherrywood Ln)	L	777	68	120	129	165	69	115	139	177
	EB (Cherrywood Ln)	R	1,304	0	65	0	200	0	62	0	194
	NB (Kenilworth Ave)	L	750	81	367	18	148	81	391	15	154
	NB (Kenilworth Ave)	Т	1,198	2	59	6	76	2	66	6	69
	SB (Edmonston Rd)	Т	594	307	301	212	204	311	306	217	198
	SB (Edmonston Rd)	R	250	31	#265	0	89	46	#285	0	83
12	Edmonston Road (MD 201)	& Sunny	side Avenu	ue (Signal	ized)						
	EB (Sunnyside Ave)	L	965	182	555	320	#1234	182	484	320	#1222
	EB (Sunnyside Ave)	R	350	332	#421	455	#425	342	#421	455	#447
	NB (Edmonston Rd)	L	450	362	387	268	#602	378	406	268	#605
	NB (Edmonston Rd)	Т	1,381	249	259	809	#1865	250	267	876	#1905
	SB (Edmonston Rd)	Т	1,554	1336	#1629	1058	#1726	1442	#1942	1058	#1647
	SB (Edmonston Rd)	R	250	23	#293	14	#336	25	#310	14	#337
13	Edmonston Road (MD 201)	& Powde	r Mill Roa	d (Signali	zed)						
	EB (Powder Mill Rd)	L	250	43	124	414	237	44	123	79	243
	EB (Powder Mill Rd)	Т	903	244	269	0	457	245	298	414	496
	EB (Powder Mill Rd)	R	500	0	83	0	154	0	96	0	191
	WB (Powder Mill Rd)	L	250	114	156	74	119	114	150	74	111
	WB (Powder Mill Rd)	Т	699	176	214	129	163	176	212	129	171
	WB (Powder Mill Rd)	R	100	0	100	0	62	0	96	0	65
	NB (Edmonston Rd)	L	400	513	364	~615	324	523	370	~679	333
	NB (Edmonston Rd)	Т	640	274	246	19	297	274	257	578	296
	NB (Edmonston Rd)	R	275	0	20	64	96	0	28	19	96
	SB (Edmonston Rd)	L	275	21	104	0	140	21	76	64	132
	SB (Edmonston Rd)	TR	822	324	301	0	310	332	307	345	300
14	Greenbelt Metro Drive & S	ite North	Access (Si	gnalized)	а						
	EB (Greenbelt Metro Dr)	Т	216	N/A	N/A	N/A	N/A	41	75	215	#283
	WB (Greenbelt Metro Dr)	L	-	N/A	N/A	N/A	N/A	-	-	-	-
	WB (Greenbelt Metro Dr)	Т	244	N/A	N/A	N/A	N/A	82	124	95	149
	NB (Site North Access)	L	234	_	-	-	-	10	51	163	#245
	NB (Site North Access)	R	234	N/A	N/A	N/A	N/A	0	32	0	115

 Table 5-22:
 Comparison of No-build to Build Condition Queuing Analysis (continued)

			Turning		No-build	Condition			Build Co	ondition	
			Turning	AM F	Peak	PM F	Peak	AM F	Peak	PM F	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)
15	Greenbelt Station Bus Bay	s/Greenb	elt Metro	Drive & Gr	eenbelt St	ation Park	way (Sigr	nalized)			
	EB (Greenbelt Sta Bus Bays	LT	216	22	59	16	54	22	60	16	58
	EB (Greenbelt Sta Bus Bays	R	-	-	-	-	-	-	-	-	-
	WB (Greenbelt Metro Dr)	L	366	412	#446	169	250	442	#465	163	253
	WB (Greenbelt Metro Dr)	Т	366	14	45	15	57	14	41	14	55
	WB (Greenbelt Metro Dr)	R	275	0	-	0	-	0	-	60	114
	NB (Greenbelt Sta Pkwy)	L	250	-	-	0	4	-	-	0	2
	NB (Greenbelt Sta Pkwy)	Т	243	100	102	50	84	129	-	64	-
	NB (Greenbelt Sta Pkwy)	R	243	31	-	12	11	45	98	30	97
16	Greenbelt Station Parkway	y & North	Core Deve	elopment/	Site North	west Acces	ss (Signali	ized)			
	EB (North Core Dev)	L	179	38	80	121	164	38	85	103	176
	EB (North Core Dev)	TR	179	0	36	0	63	0	36	0	100
	WB (Site Northwest Access)	LTR (AM)	-	-	-	-	-	0	53	-	-
	WB (Site Northwest Access)	LT (PM)	-	-	-	-	-	-	-	-	-
	WB (Site Northwest Access)	R (PM)	-	-	-	-	-	-	-	267	#265
	NB (Greenbelt Sta Pkwy)	L	505	28	197	33	131	56	183	44	137
	NB (Greenbelt Sta Pkwy)	TR	505	28	107	67	228	78	166	87	206
	SB (Greenbelt Sta Pkwy)	TR	266	0	22	0	13	0	23	0	13
17	Greenbelt Station Parkway	y & Resid	ential Acc	ess to 500	Units (TW	SC)					
	EB (Residential Access)	R	174	-	59	-	49	-	63	-	51
	NB (Greenbelt Sta Pkwy)	Т	465	-	3	-	302	-	7	-	229
18	Greenbelt Station Parkway	y & I-95/I-4	495 Off-rar	nps/Site S	outh Acce	ss/Kiss & R	Ride (Sign	alized)			
	EB (I-95 Off-ramps)	L	229	238	223	187	134	~797	#2534	196	164
	EB (I-95 Off-ramps)	LTR	229	129	222	21	153	~666	#2456	30	155
	EB (Kiss and Ride)	L	188	229	#258	116	174	229	#262	117	179
	WB (Site South Access)	R	407	6	27	118	160	7	25	118	176
	NB (Greenbelt Sta Pkwy)	L	375	24	59	35	76	24	56	35	78
	NB (Greenbelt Sta Pkwy)	Т	530	325	86	110	87	356	87	111	88
	SB (Greenbelt Sta Pkwy)	L	400	0	120	0	54	176	172	0	62
	SB (Greenbelt Sta Pkwy)	TR	465	0	73	28	93	3	77	28	83

 Table 5-22:
 Comparison of No-build to Build Condition Queuing Analysis (continued)

			Turning		No-build	Condition			Build Co	ondition	
	Intersection and	Lane	Turning Bay/Link	AM F	Peak	PM F	Peak	AM F	Peak	PM	Peak
#	Approach	Group	Length	50th	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)
19	Greenbelt Station Parkway	y & WMA <sup>.</sup>	TA Garage	e (Signaliz	ed)						
	EB (WMATA Garage)	L	150	7	30	100	#158	7	29	100	#162
	EB (WMATA Garage)	R	290	0	24	0	63	0	23	0	85
	NB (Greenbelt Sta Pkwy)	LT	330	358	183	157	80	409	176	157	81
	NB (Greenbelt Sta Pkwy)	TR	330	4	145	48	99	4	140	48	88
	SB (Greenbelt Sta Pkwy)	Т	162	141	68	248	152	161	58	240	155
	SB (Greenbelt Sta Pkwy)	R	162	23	14	0	2	32	11	0	2
20	Greenbelt Station Parkway	y & Resid	ential Acc	ess to 300	Units (TW	SC)					
	EB (Residential Access)	LR	224	-	64	-	44	-	60	-	42
	NB (Greenbelt Sta Pkwy)	LT	345	-	0	-	0	-	0	-	0
	SB (Greenbelt Sta Pkwy)	TR	350	-	5	-	6	-	-	-	5
21	Greenbelt Road (MD 193) 8	& Greenb	elt Station	Parkway	(Signalize	d)				_	
	EB (Greenbelt Rd)	L	67	95	144	97	#142	124	180	98	#144
	EB (Greenbelt Rd)	Т	1,008	84	95	360	233	84	89	360	227
	WB (Greenbelt Rd)	Т	1,584	117	130	165	199	116	213	173	198
	WB (Greenbelt Rd)	R	150	0	71	19	#167	2	73	20	#177
	SB (Greenbelt Sta Pkwy)	L	524	115	162	125	185	115	151	125	191
	SB (Greenbelt Sta Pkwy)	R	225	165	209	184	#242	167	208	185	#235

 Table 5-22:
 Comparison of No-build to Build Condition Queuing Analysis (continued)

Notes:

~ 50th percentile volume exceeds capacity, queue is theoretically infinite.

# 95th percentile volume exceeds capacity, queue may be longer.

m Volume for 95th percentile queue is metered by upstream signal. Due to upstream metering, the 95th percentile queue may be less than the 50th percentile queue.

AWSC = All-way STOP-Controlled intersection

EB = Eastbound, WB = Westbound, NB= Northbound, SB = Southbound

LTR = left / through / right lanes

TWSC = Two-way STOP-Controlled intersection

Red cells denote approaches and lane groups whose queuing length exceeds capacity.

<sup>a</sup> Intersection would be included under the Build Condition, but was included as part of the No-build Condition design provided by Renard Development Company, LLC.

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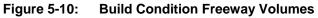
### 5.8.7 Overall Traffic Assessment

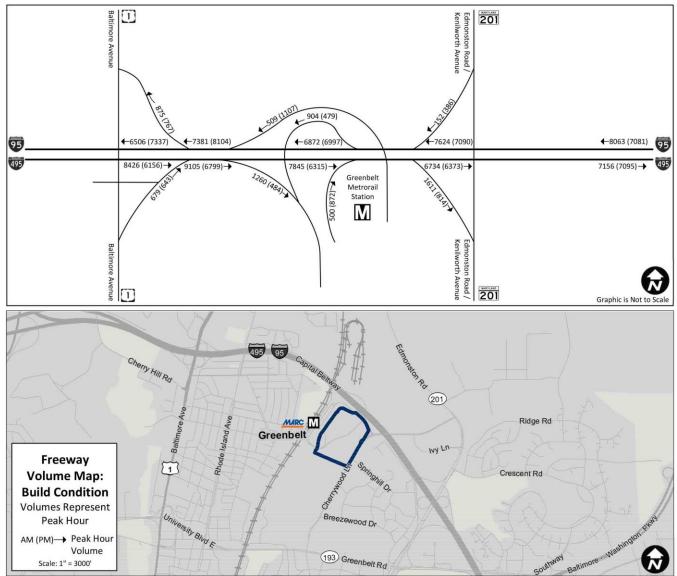
Overall, the PM peak hour would experience isolated intersection impacts at the Edmonston Road (MD 201) at Powder Mill Road, Edmonston Road (MD 201) and Sunnyside Avenue, and Cherrywood Lane and Ivy Lane intersection (Ivy Lane approaches only). Together these conditions would result in direct, long-term, adverse impacts at intersections.

Because the intersections along Edmonston Road at Sunnyside Avenue and Powder Mill Road are forecasted to be failing during the No-build Condition, adding construction-related trips along this route caused by trucks, employees, and equipment would result in isolated impacts. These conditions would result in direct, short-term, adverse impacts during the construction period.

### 5.8.8 Build Condition Freeway Volumes

Although freeway analysis was not performed for the Build Condition, freeway ramp volumes are included in figure 5-10 to allow a comparison to the Existing Condition, No-build Condition, and Build with Mitigation Condition freeway ramp volumes presented in Sections 3.7, 4.8, and 6.6, respectively. Full analysis of the freeway volumes is included in the Build with Mitigation Condition in Section 6.6.





### 5.9 Transportation Demand Management

TDM is a set of strategies, programs, services, and physical elements that influence travel behavior by mode, frequency, time, route, or trip length in order to help achieve highly efficient and sustainable use of transportation facilities (DDOT 2010, p.5). TDM measures for the Greenbelt Build with Mitigation Condition would be developed as part of the Final EIS if the Greenbelt site is chosen as the preferred consolidation location for the FBI HQ. The TDM measures would encourage the reduction of SOV trips by "focusing the demand for transportation services on alternative modes and providing the public with the incentives as well as information to use these alternatives."

The introduction of TDM measures would serve to ensure the transportation mode splits planned in this study were achieved as well as serve to mitigate travel mode, frequency, time, route, and/or trip length associated with future trips of the consolidated FBI HQ.

# 6.0 Mitigation Measures

To reduce impacts on the transportation system caused as a result of the proposed action—consolidation of the FBI HQ at the Greenbelt site, mitigation measures are recommended in this section for each mode of transportation analyzed. Also included is a sample of Transportation Demand Management (TDM) measures to encourage non-SOV travel. Overall, the Greenbelt site requires moderate mitigation to reduce direct impacts of the proposed action.

### 6.1 Pedestrian Network

No pedestrian mitigation is necessary under the Greenbelt Build with Mitigation Condition because any pedestrian improvements outside of the Greenbelt site would be built as part of the Greenbelt Station development project.

When compared to the Build Condition, there would be no difference in the long-term or short-term pedestrian network impacts under the Build with Mitigation Condition, because the recommended mitigation measures would not change the proposed pedestrian network. Therefore, under the Build with Mitigation Condition, there would continue to be direct, long-term, beneficial impacts to the pedestrian network and no measureable direct, shirt-term impacts to the pedestrian network because of construction.

### 6.2 Bicycles

No mitigation is recommended for the bicycle network in the study area. The site currently has adequate bicycle facilities on Greenbelt Metro Drive and Cherrywood Lane, along with a connection to Lackawanna Street on the west side of the Greenbelt Metro Station. The proposed bicycle lanes on Greenbelt Station Parkway, to be built as part of the No-build Condition, would augment the existing network of bicycle facilities around the site.

When compared to the Build Condition, there would be no difference in the long-term or short-term bicycle network impacts under the Build with Mitigation Condition, because the recommended mitigation measures would not change the proposed bicycle network. Therefore, under the Build with Mitigation Condition, there would continue to be no measurable direct, long-term or short-term impacts to the study area bicycle network from the proposed action or from construction, respectively.

### 6.3 Public Transit

No public transit mitigation is necessary under the Greenbelt Build with Mitigation Condition.

When compared to the Build Condition, there would be no difference in long-term public transit capacity impacts under the Build with Mitigation Condition. Therefore, there would continue to be no measureable direct, long-term impacts to public transit capacity. However, the bus operation delays along Edmonston Road (three bus routes) would be improved resulting in direct, long-term beneficial impacts. During construction, when compared to the Build Condition, there would be worse public transit impacts under the Build with Mitigation Condition, changing from direct, short-term, adverse impacts to direct, short-term, major adverse impacts caused by construction vehicles blocking some or all of the lanes on the road and intermittent road closures along Edmonston Road and 60th Avenue.

### 6.4 Parking

As mentioned in the Build Condition section, parking impacts would largely be addressed through development and implementation of a Transportation Management Plan (TMP), which would include preferred strategies for discouraging employees from parking on local streets. Because the TMP would be implemented as part of the Build Condition, there would be no changes in parking impacts between the Build and Build with Mitigation Conditions.

When compared to the Build Condition, there would be no change in long-term parking impacts; therefore there would continue to be no measurable direct, long-term impacts to parking. Compared to the Build Condition, there would be no difference in the short-term construction parking impacts under the Build with Mitigation Condition. There would continue to be no measurable direct, short-term parking impacts because the recommended mitigation measures would not impact parking during the construction period.

## 6.5 Truck Access

No mitigation is recommended for truck access. Note that the Build Condition includes proper signing and communication of truck access restrictions to alleviate impacts to truck access.

When compared to the Build Condition, there would be no difference in the long-term or short-term truck access impacts under the Build with Mitigation Condition, because the measures would not change the truck access conditions. Therefore, there would continue to be no measurable direct, long-term or short-term truck access impacts during operation of the facility or during construction, respectively.

### 6.6 Traffic Analysis

### 6.6.1 Development of Mitigated Network

Based on the Build Condition traffic operations and queueing analysis, most of the intersections would not fail (defined in the Section 3.7.1, *Existing Condition* section) or require mitigation; therefore a second DTA was not necessary. In addition, the principal decision point would be at the Greenbelt Station Parkway and I-95 off-ramp where FBI inbound vehicles would either drive straight into the Site South Access or turn left and enter through the Site Northwest Access. The DTA provided a balanced vehicle flow between the two ECFs and would be initially used to measure the ECF queuing impacts.

### 6.6.2 Recommended Mitigation Measures

Based on the DTA results from the Build Condition, the resulting traffic volumes were entered into Synchro<sup>™</sup> to determine the study area intersection operations and queuing. Each intersection that had LOS degradation from a passing LOS (A-D) to a failing LOS (E or F) by lane group (right turns, through movements, or left turns) when compared to the No-build Condition was mitigated by one of the following methods:

- Optimizing the existing traffic signal (change the amount of seconds of green to each approach)
- Coordinating a corridor of traffic signals
- Revising the existing lane geometry (number of right versus through versus left-turning lanes)
- Adding new turning lanes
- Adding through lanes

Because M-NCPPC requires each intersection to be analyzed based on the CLV method, each of the intersection geometry and Build with Mitigation vehicle volumes were entered into the CLV worksheet to ensure the proposed recommended mitigation also resulted in a passing CLV LOS. Similar to the HCM method, M-NCPPC requires that failing intersections be improved to better condition than the No-build Condition (the difference between the failing CLV and mitigated CLV must be reduced by at least 150 percent of the delta between the failing CLV and No-build CLV).

A list of mitigation measures was developed through an iterative process of testing the different improvement strategies, starting with optimizing the traffic signals and progressing to adding lanes if warranted. The recommended roadway improvements include external roadway mitigation measures that support the revised Greenbelt conceptual site plan. If implemented, the external roadway mitigations would improve the traffic operations at all study area intersections to a passing LOS (both HCM-based and CLV-based) or, if failing, would be equal to or better than the No-build Condition operations. The recommended mitigations would also result in no vehicle queues beyond the available storage capacity, or if beyond the storage capacity, would be no greater than 150 feet longer than the queues measured for the No-build Condition. An acceptable queue length increase is not cited in the VDOT Traffic Impact Analysis Regulations; therefore, the 150 feet is referenced from the DDOT Comprehensive Transportation Review Requirements guidance and provides a reasonable increase (approximately 6 vehicles or less) (DDOT 2012).

Table 6-1 shows the locations of the mitigation measures. Figure 6-1 shows the locations of the mitigation measures, figure 6-2 shows the lane geometry with the mitigation in place, and figure 6-3 shows the AM peak hour inbound and PM peak hour outbound FBI vehicle trip paths.

Map ID	Mitigation	Strip Land Taking (Approximate Linear Feet)				
Α	Edmonston Road (MD 201) and Powder Mill Road					
<ul> <li>For the Edmonston Road northbound approach, create a new 400-foot left-turn lane and lengthen the right turn-lane by 50 feet resulting in a 325-foot right-tune lane, resulting in two left-turn lanes, one through lane, and one right-turn lane.</li> <li>Extend the existing northbound left-turn lane back to the previous intersection at Sunnyside Road resulting in widening the northbound direction by one lane.</li> <li>Add a second departing lane totaling approximately 700 feet along westbound Powder Mill Road resulting in two westbound travel lanes for 700 feet,</li> <li>Optimize the traffic signal for AM and PM peak periods.</li> </ul>						
В	Edmonston Road (MD 201) and Sunnyside Road					
450 lane • For 600 • Add Edm	the Edmonston Road northbound approach, create a new through lane extending back feet to match the left-turn lane distance resulting in one left-turn lane and two through es. the Edmonston Road southbound approach, create a new through lane extending back feet resulting in two through lanes and one right-turn lane. a second departing lane totaling approximately 1,500 feet along southbound nonston Road resulting in two southbound travel lanes for 1,500 feet. imize the traffic signal for AM and PM peak periods.	2,550				
С	Greenbelt Road (MD 193) and Cherrywood Lane/60th Avenue					
turn • Opt	the 60th Avenue northbound approach, create a new 120-foot lane resulting in one left- lane and one shared through/right turn lane. imize the traffic signal for AM and PM peak periods and coordinate timings with nearby intersections for AM and PM peak periods.	None				

#### **Table 6-1: Recommended Mitigation Measures**

#### Table 6-1: Recommended Mitigation Measures (continued)

Map ID	Mitigation	Strip Land Taking (Approximate Linear Feet)
D	Greenbelt Road (MD 193) and Greenbelt Station Parkway	
• Coc	rdinate timings with nearby key intersections for the AM peak period.	None
E	Greenbelt Station Parkway and WMATA Garage	
	mize the traffic signal for AM and PM peak periods and coordinate timings with nearby intersections for AM and PM peak periods.	None
F	Greenbelt Station Parkway and I-95/I-495 off-ramp/Site South Access	S
imp exis • Opt	the Greenbelt Metro Station Kiss & Ride approach, revise the planned roadway rovement design to include a second lane totaling 200 feet (50 feet more if space ts). mize the traffic signal for AM and PM peak periods and coordinate timings with nearby intersections for AM and PM peak periods.	None
G	Greenbelt Station Parkway and North Core Mixed Use/Site Northwest Ac	cess
	mize the traffic signal for AM and PM peak periods and coordinate timings with nearby intersections for AM and PM peak periods.	None
Н	Greenbelt Station Parkway and Greenbelt Metro Drive	
-	mize the traffic signal for AM and PM peak periods and coordinate timings with nearby intersections for AM and PM peak periods.	None
l.	Greenbelt Metro Drive and Site North Access	
Ado     Driv	all a traffic signal. a second departing lane approximately 500 feet along westbound Greenbelt Metro e connecting into the left-turn lane at the next intersection. mize the traffic signal for AM and PM peak periods.	None
J	I-95/I-495 off-ramp from the Interstates to Greenbelt Station Parkway	
lead Gre	ise the planned roadway improvement design to stripe the exit ramp for the right lane to I directly into the WMATA Garage, the center lane to lead to the right lane at the enbelt Station Parkway intersection, and the left lane to service the Kiss & Ride and ter and left lanes at the Greenbelt Station Parkway intersection.	None

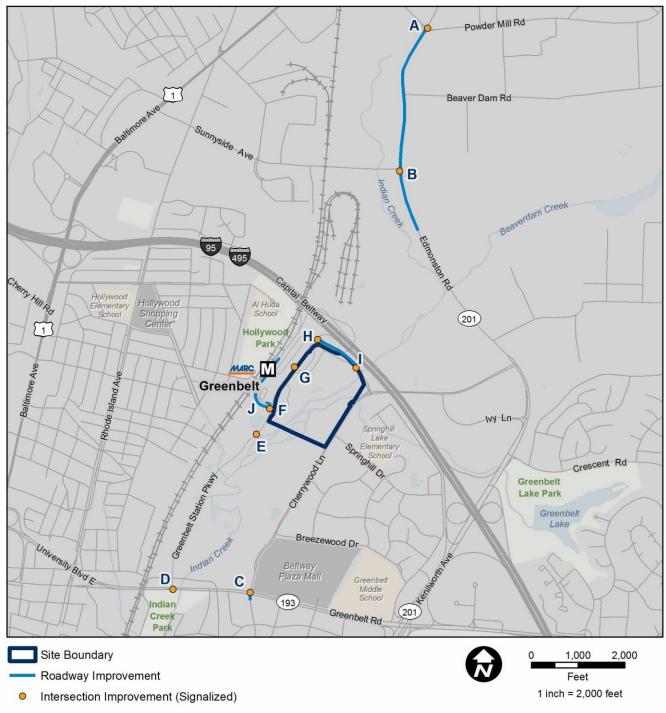


Figure 6-1: Build with Mitigation Condition Improvement Locations

Sources: ESRI (2013), GSA (2013) Prince George's County (2013) (This page intentionally left blank.)







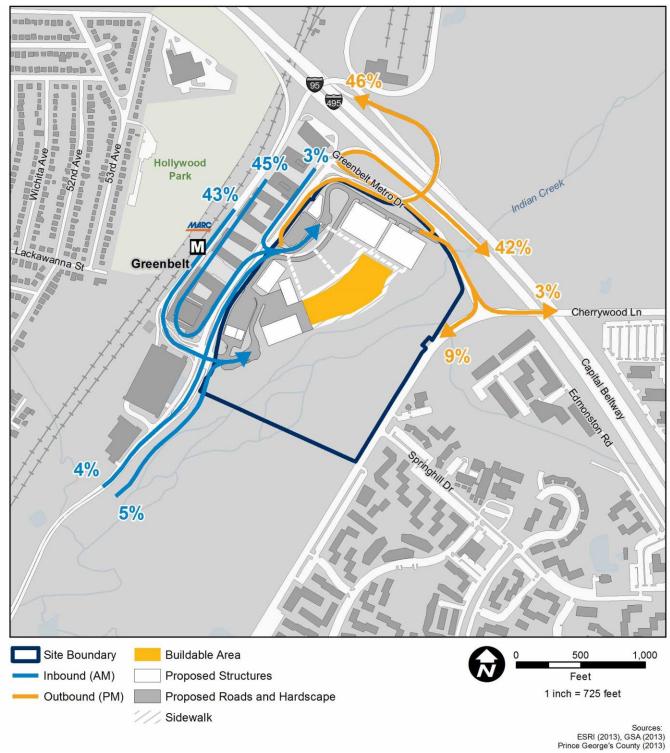


Figure 6-3: AM Peak Hour Inbound and PM Peak Hour Outbound FBI Vehicle Trip Paths

### 6.6.3 Land Use Impact Summary

This section references the Prince George's County internet-based PGAtlas tool to provide an estimate of property impacts (M-NCPPC 2012b). Several of the proposed recommended mitigation measures might require property strip takings at two intersections: Edmonston Road at Sunnyside Avenue and Edmonston Road and Powder Mill Road. The Edmonston Road and Sunnyside Avenue intersection mitigation measures would impact the northbound direction beginning 450 feet south of the intersection and continuing 2,950 feet north leading into the intersection at Powder Mill Road. Measures would also include a new lane added to the southbound direction beginning 600 feet north of the intersection and continuing 2,100 feet south.

The Edmonston Road at Powder Mill Road mitigation measures would impact the northbound approach and westbound departing segments. The northbound approach impact would include 400 feet as part of second left-turn lane, and the westbound departing segment would include a 200-foot stretch where the county right-of-way ownership line narrows bordering on the edge of the existing pavement.

There would be four parcels impacted, all tax-exempt-status properties. One property is privately owned, and the other three properties are federally owned. Note that these potential impacts are based on conceptual roadway changes. During the design phase, the property impacts would be more defined to identify the total square acres impacted and design measures that could be employed to lessen the impact, such as narrowing travel lanes or shifting the roadway alignment.

### 6.6.4 Intersection Operations Analysis

Synchro<sup>™</sup> was used to calculate the vehicle delay and LOS operation based on the HCM 2000 method for each study area intersection. Custom designed Excel sheets were used to calculate the LOS operation based on the CLV method. Based on the Synchro<sup>™</sup> and CLV-based Excel worksheet analysis,

#### 6.6.4.1 Signalized Intersection Operations Analysis

Based on the Synchro<sup>™</sup> and CLV-based Excel worksheet analysis, all but one signalized study area intersection would operate at acceptable overall conditions during the morning and afternoon peak hours. The following intersection in the study area would operate with overall unacceptable conditions, which include LOS E or LOS F using HCM 2000 method or LOS F using the CLV method:

 Edmonston Road (MD 201) and Powder Mill Road (Intersection #13) would operate at CLV F during the PM peak hour

Note that the Build with Mitigation Condition would result in a better operate than the Nobuild Condition. The Build with Mitigation Condition CLV would decrease when compared to the No-build Condition CLV by a CLV of 224, a 12 percent decrease.

Compared to No-build Condition, one fewer intersection would fail overall, resulting in one failure in the PM peak hour. Based on the Synchro<sup>™</sup> analysis, there would be no signalized intersection that would have lane groups or overall approaches with LOS degradation from an acceptable condition (LOS A through LOS D) to an unacceptable condition (LOS E or LOS F) when compared to the No-build Condition during the morning or afternoon peak hours.

#### 6.6.4.2 Unsignalized Intersection Operations Analysis

Based on the Synchro<sup>™</sup> analysis, the following two unsignalized intersections would have lane groups or overall approaches with LOS degradation from an acceptable condition (LOS A through LOS D) to an unacceptable

condition (LOS E or LOS F) when compared to the No-build Condition during the morning or afternoon peak hours:

 Cherrywood Lane and Springhill Drive (Intersection #3) would result in the Springhill Drive (minor approach) operating at a worse LOS F than the No-build Condition (average control delay would increase by 47.9 seconds)

Note that this intersection was analyzed using the CLV method and resulted in a CLV of 893; therefore, the intersection does not require further study (based on the Prince George's County Guidelines outlining a second test for HCM-based failing unsignalized intersection approaches where the CLV-based method should be analyzed to determine if the results are less than CLV of 1,150).

 Cherrywood Lane and Ivy Lane (Intersection #5) would result in the Ivy Lane (minor approach) operating at a worse LOS F than the No-build Condition (average control delay would increase by 40.9 seconds)

Note that this intersection was analyzed using the CLV method and resulted in a CLV of 1,115; therefore, the intersection does not require further study (based on the Prince George's County Guidelines outlining a second test for HCM-based failing unsignalized intersection approaches where the CLV-based method should be analyzed to determine if the results are less than a CLV of 1,150).

 Note that the minor street failing traffic operations are due to the proposed Capital Office Park development located north of Cherrywood Lane, which elevated the operations from passing during the Existing Condition to failing operations during the No-build Condition.

#### 6.6.4.3 Complete Intersection Operations Analysis

The average LOS for the various approaches to the intersections and the overall intersection LOS grades for the Build with Mitigation Condition are depicted in figures 6-4 and 6-5 for the AM and PM peak hours, respectively. Table 6-1 shows the results of the LOS capacity analysis and the intersection projected delay under the No-build Condition compared to the Build with Mitigation Condition during the AM and PM peak hours.

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#### Figure 6-4: Build with Mitigation Condition Intersection LOS for AM Peak Hour

Note: One- or two-way STOP-Controlled unsignalized intersections do not have an overall intersection LOS value, since the mainline through move operates freely through the intersection. Red shaded circles denote intersections/approaches operating at LOS E or F. [\*] = Unsignalized intersection requires attention due to failing minor approach movements.



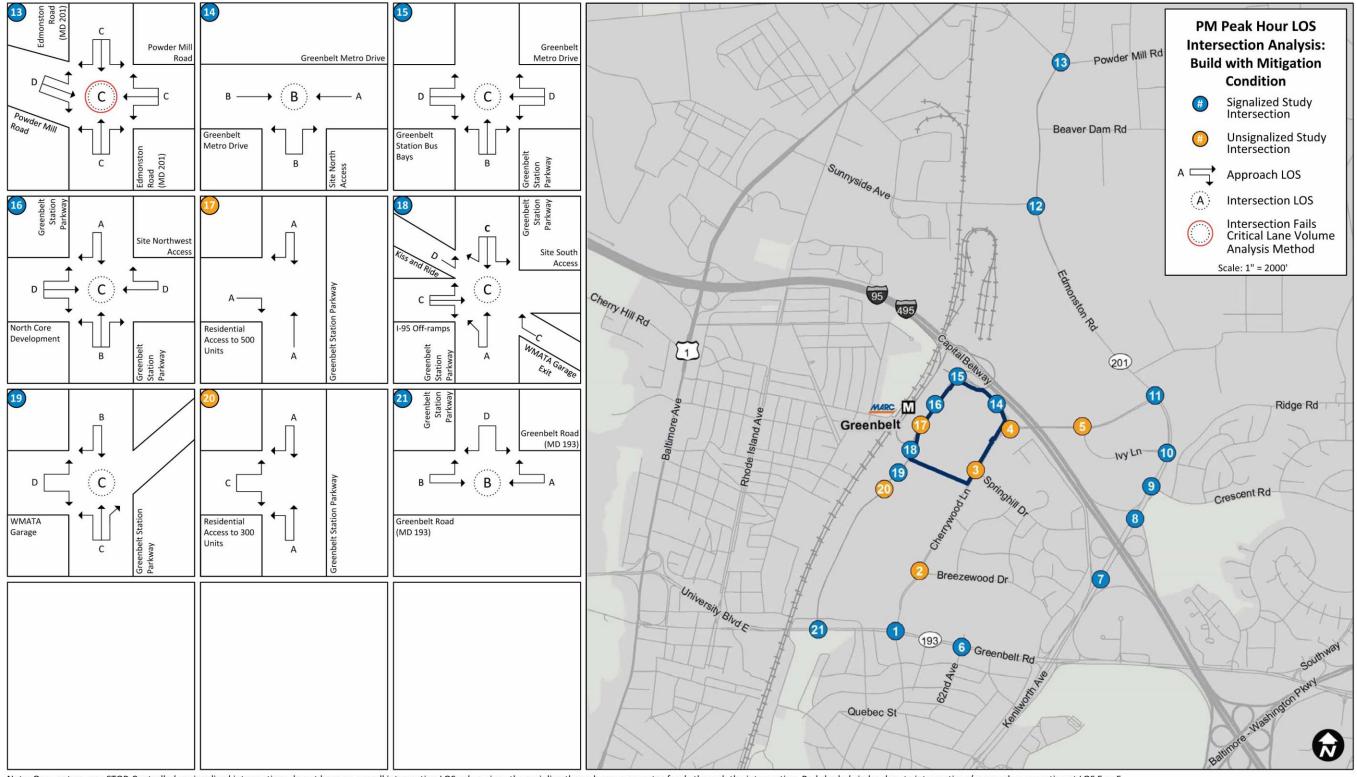


Note: One- or two-way STOP-Controlled unsignalized intersections do not have an overall intersection LOS value, since the mainline through move operates freely through the intersection. Red shaded circles denote intersections/approaches operating at LOS E or F. [\*] = Unsignalized intersection requires attention due to failing minor approach movements.



#### Build with Mitigation Condition Intersection LOS for PM Peak Hour Figure 6-5:

Note: One- or two-way STOP-Controlled unsignalized intersections do not have an overall intersection LOS value, since the mainline through move operates freely through the intersection. Red shaded circles denote intersections/approaches operating at LOS E or F. [\*] = Unsignalized intersection requires attention due to failing minor approach movements.



Note: One- or two-way STOP-Controlled unsignalized intersections do not have an overall intersection LOS value, since the mainline through move operates freely through the intersection. Red shaded circles denote intersections/approaches operating at LOS E or F. [\*] = Unsignalized intersection requires attention due to failing minor approach movements.

Table 6-1:	Comparison of No-build and	Build with Mitigation Condition	n Intersection AM and PM Peak Hou	r Operations Analysis
		· · · · · · · · · · · · · · · · · · ·		

						No-	build	Condit	tion						Buil	d wit	h Mitig	ation	Cond	ition		
				AM	Peak H	Hour			PM	Peak F	lour			AM	Peak H	lour			ΡM	Peak H	lour	
#	Intersection and Approach	Lane Group	нсм	2000	CL		Chaole	нсм	2000	CL	v	Chaste	HCM :	2000	CL	v	Chask	HCM	2000	CL	v	Chash
			Delay (sec/ veh)	LOS	Critical Lane Vol		Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol		Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check
1	Greenbelt Road (MD 193) & C	Cherrywo	ood La	ne/60	0th Ave	nue (S	Signal	ized)										_				
	EB (Greenbelt Rd)	L	63.2	E				53.0	D				59.4	Е				44.9	D			
	EB (Greenbelt Rd)	TR	8.8	Α	]			13.9	В				7.5	А				16.5	В			
	EB Overall (Greenbelt Rd)		19.1	В				21.2	С				17.3	В				21.8	С			
	WB (Greenbelt Rd)	L	64.2	Е				67.0	Е				64.6	Е				56.6	Ш			
	WB (Greenbelt Rd)	TR	20.6	С				35.7	D				19.8	В				40.7	D			
	WB Overall (Greenbelt Rd)		21.5	С	]			36.9	D				20.7	С				41.3	D			
	NB (60th Ave)	L	-	-	]			-	-				71.6	Е				78.3	Е			
	NB (60th Ave)	LTR/TR	74.0	Е				132.4	F				70.3	Е				93.9	F			
	NB Overall (60th Ave)		74.0	E				132.4	F				70.8	E				88.6	F			
	SB (Cherrywood Ln)	L	76.7	Е				106.8	F				71.5	Е				102.7	F			
	SB (Cherrywood Ln)	LT	76.7	Е				108.0	F				71.8	Е				102.9	F			
	SB (Cherrywood Ln)	R	70.0	Е				83.5	F				69.4	Е				77.1	Е			
	SB Overall (Cherrywood Ln)		71.9	E				91.0	F				70.0	Е				85.4	F			
	Overall		28.5	С	1,315	D	Pass	42.2	D	1,504	E	Pass	27.1	С	1,283	С	Pass	42.4	D	1,501	E	Pass
2	Cherrywood Lane & Breezew	vood Dri	ive (AV	VSC)	_	-				-					_							
	WB (Breezewood Dr)	LR	13.3	-				12.5	-				13.4	-				13.0	-			
	WB Overall (Breezewood Dr)	)	13.3	В				12.5	В				13.4	В				13.0	В			
	NB (Cherrywood Ln)	Т	11.2	-				12.4	-				11.3	-				12.9	-			
	NB (Cherrywood Ln)	R	8.7	-				9.4	-				8.7	-				9.7	-			
	NB Overall (Cherrywood Ln)		10.1	В	1			11.1	В				10.2	В				11.5	В			
	SB (Cherrywood Ln)	L	9.7	-	1			10.5	-				9.7	-				10.6	-			
	SB (Cherrywood Ln)	Т	10.8	-	1			15.1	-				11.0	-				21.9	-			
	SB Overall (Cherrywood Ln)		10.4	В	1			13.7	в	-			10.6	В				19.0	С			
	Overall		11.2	В	N/A	N/A	Pass	12.5	В	N/A	N/A	Pass	11.3	В	N/A	N/A	Pass	15.2	С	N/A	N/A	Pass
3	Cherrywood Lane & Springhi	ill Drive	(TWS	C)																	·	
	WB (Springhill Dr)	LR	16.4	Ċ				128.6	F				16.5	С				176.5	F			
	WB Overall (Springhill Dr)		16.4	С	1			128.6					16.5	С				176.5	F			
	SB (Cherrywood Ln)	L	8.3	A	1			8.7	Α				8.3	A				8.7	A			
	SB Overall (Cherrywood Ln)		3.0	-	1			2.4	-				3.0	_				2.1	_			
	Overall		5.2	_	N/A	N/A	Pass	27.0	-	N/A	N/A	Pass	5.2		N/A	N/A	Pass	34.3		N/A	N/A	Pass

Table 6-1:	Comparison of No-build and Build with Mitig	ation Condition Intersection AM and PM Peak Hou	r Operations Analysis (continued)

						No-	-build	Condit	ion						Buil	d witl	h Mitig	ation	Cond	ition		
				AM	Peak H	lour			PM	Peak F	lour			AM	Peak H	lour			PM	Peak H	lour	
#	Intersection and Approach	Lane Group	НСМ	2000	CL			HCM	2000	CL			HCM :	2000	CL	v		HCM :	2000	CL	v	
			Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Chec
4	Cherrywood Lane & Greenbe	elt Metro	Drive	(Rou	ndabou	ut) <sup>a</sup>				-	-									-		
	EB (Greenbelt Metro Dr)	LR	6.1	Α				14.6	В				6.2	А				16.5	С			
	EB Overall (Greenbelt Metro	Dr)	3.3	Α				7.5	Α				3.3	Α				7.9	Α			
	NB (Cherrywood Ln)	LT	11.8	В				14.4	В				11.8	В				15.6	С			
	NB Overall (Cherrywood Ln)		11.8	В				14.4	В				11.8	В				15.6	С			
	SB (Cherrywood Ln)	Т	6.3	Α				12.0	В				6.3	А				12.0	В			
	SB Overall (Cherrywood Ln)		2.2	Α				8.9	Α	1			2.0	Α				8.9	Α			
	Overall		6.0	Α	N/A	N/A	Pass	9.8	Α	N/A	N/A	Pass	5.8	Α	N/A	N/A	Pass	10.0	В	N/A	N/A	Pass
5	Cherrywood Lane & Ivy Lane	(TWSC	)																			
	EB (Cherrywood Ln)	LTR	3.0	Α				0.4	А				3.1	А				0.4	А			
	EB Overall (Cherrywood Ln)		3.0	-				0.4	-	1			3.1	-				0.4	-			
	WB (Cherrywood Ln)	L	8.3	Α				8.8	А				8.3	А				8.9	А			
	WB (Cherrywood Ln)	TR	0.0	-				0.0	-	1			0.0	-				0.0	-			
	WB Overall (Cherrywood Ln)		0.4	-				0.2	-	1			0.4	-				0.2	-			
	NB (lvy Ln)	LT	67.2	F				Λ	F				79.7	F				^	F			
	NB (lvy Ln)	R	10.3	В				12.1	В				10.3	В				12.5	В			
	NB Overall (Ivy Ln)		55.7	F				Λ	F				65.7	F				^	F			
	SB (lvy Ln)	LTR	41.0	Е				402.7	F				44.7	Е				443.6	F			
	SB Overall (Ivy Ln)		41.0	Е				402.7	F				44.7	Е				443.6	F			
	Overall		6.0	-	N/A	N/A	Pass	^	-	N/A	N/A	Fail	6.6	-	N/A	N/A	Pass	۸	-	N/A	N/A	Fail
6	Greenbelt Road (MD 193) & 6	2 Avenu	e/Belt	way I	Plaza D	rivew	vay (Si	gnaliz	ed)													
	EB (Greenbelt Rd)	L	1.7	A				7.0	Á				2.7	А				8.3	А			
	EB (Greenbelt Rd)	TR	2.6	А				11.3	В	1			3.5	А				22.2	С			
	EB Overall (Greenbelt Rd)		2.6	Α				11.2	В	1			3.5	Α				22.0	С			
	WB (Greenbelt Rd)	L	4.0	Α				24.7	С	1			4.1	А				25.4	С			
	WB (Greenbelt Rd)	Т	7.5	Α				18.3	В	1			7.6	А				18.3	В			
	WB (Greenbelt Rd)	R	4.7	Α				14.8	В	1			4.7	А				14.8	В			
	WB Overall (Greenbelt Rd)		7.2	Α				17.8	В	1			7.4	Α				17.8	в	1		
	NB (62th Ave)	LTR	68.1	Е				71.4	Е				68.1	Е				71.4	Е			
	NB Overall (62th Ave)		68.1	Е				71.4	Е				68.1	Е				71.4	Е			
	SB (Beltway Plaza Drwy)	L	68.2	Е				69.8	Е				68.2	Е				69.8	Е			
	SB (Beltway Plaza Drwy)	LT	68.3	Е				69.5	Е				68.3	Е				69.5	Е			
	SB (Beltway Plaza Drwy)	R	66.7	Е				54.9	D				66.7	Е				54.9	D			
	SB Overall (Beltway Plaza D		67.8	Е				67.1	Е	1			67.8	Е				67.1	Е			
	Overall		7.5	Α	742	Α	Pass		С	1,206	С	Pass	7.9	А	757	Α	Pass		С	1,220	С	Pass

Table 6-1:	Comparison of No-build and Build with Mitigation Condition Intersection AM and PM Peak Hour Operations Analysis (continue	ed)

					No	-build	Condit	tion						Buil	d witl	h Mitig	ation	Cond	ition		
			AM	Peak H	lour			PM	Peak H	lour			AM	Peak H	lour			PM	Peak H	lour	
# Intersection and Approach	Lane Group	НСМ	2000	CL	V		НСМ	2000	CL	/	Olympik	HCM 2	2000	CL۱		Ohaala	HCM	2000	CL	v	
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Checl
7 Kenilworth Avenue (MD 201)	) & I-95/I-	495 SE	B Off-r	amp (S	ignal	ized)	-										-		-		
EB (I-95/I-495 SB Off-ramp)	L	39.7	D				39.7	D				39.7	D				39.7	D			
EB (I-95/I-495 SB Off-ramp)	R	6.9	А				0.6	Α				7.0	А				0.6	А			
EB Overall (I-95/I-495 SB Off-	-ramp)	13.8	В				14.9	В				13.9	В				14.7	В			
NB (Kenilworth Ave)	Т	4.0	А				4.0	Α				4.0	А	1			4.0	А			
NB Overall (Kenilworth Ave)	)	4.0	Α				4.0	Α				4.0	Α				4.0	Α			
SB (Kenilworth Ave)	Т	6.2	А				3.6	Α				6.2	А				3.6	А			
SB Overall (Kenilworth Ave)	)	6.2	Α				3.6	Α				6.2	Α				3.6	Α			
Overall		9.1	Α	730	Α	Pass	6.8	Α	593	Α	Pass	9.1	Α	730	Α	Pass	6.8	Α	594	Α	Pass
8 Kenilworth Avenue (MD 201)	) & I-95/I-	495 NE	B Off-r	amp (S	ignal	ized)			•			I		<u>e</u>						<u></u>	
WB (I-95/I-495 NB Off-ramp)	L	24.6	С				34.3	С				24.5	С				34.3	С			
WB (I-95/I-495 NB Off-ramp)	R	26.3	С				31.1	С				26.2	С				31.1	С			
WB Overall (I-95/I-495 NB Of	f-ramp)	25.4	С				32.8	С				25.3	С				32.8	С			
NB (Kenilworth Ave)	T	11.1	В				5.4	А				11.2	В				5.4	А			
NB Overall (Kenilworth Ave)	)	11.1	В				5.4	Α				11.2	В				5.4	Α			
SB (Kenilworth Ave)	Т	7.7	А				3.4	Α				7.8	А				3.3	А			
SB Overall (Kenilworth Ave)	)	7.7	Α				3.4	Α				7.8	Α				3.3	Α			
Overall		16.7	В	868	Α	Pass	13.3	в	779	Α	Pass	16.7	в	868	Α	Pass	13.3	В	781	Α	Pass
9 Kenilworth Avenue (MD 201)	)&Cresc	ent Ro	ad/M	aryland	SHA	Office	e (Sign	alize	d)					8						<u> </u>	<u>.</u>
EB (Maryland SHA Office)	LTR	26.0	С				36.1	D				26.0	С				36.1	D			
EB Overall (Maryland SHA C	Office)	26.0	С				36.1	D				26.0	С				36.1	D			
WB (Crescent Rd)	LT	43.2	D				47.8	D				43.2	D				47.8	D			
WB (Crescent Rd)	R	26.6	С				36.3	D				26.6	С				36.3	D			
WB Overall (Crescent Rd)	•	38.0	D				43.0	D				38.0	D				43.0	D			
NB (Kenilworth Ave)	L	47.4	D				61.5	Е				47.3	D				61.5	Е			
NB (Kenilworth Ave)	Т	13.3	В				10.4	В	1			13.4	В				10.4	В			
NB (Kenilworth Ave)	R	8.5	Α				5.9	Α	1			8.6	А				5.9	А			
NB Overall (Kenilworth Ave)	)	13.9	В				10.2	В	1			14.0	В				10.2	В			
SB (Kenilworth Ave)	L	67.1	Е				53.3	D	1			67.0	Е				53.8	D			
SB (Kenilworth Ave)	Т	4.7	А				5.8	Α	1			4.7	А				5.8	Α			
SB (Kenilworth Ave)	R	12.0	В				4.9	Α	1			12.0	В				4.9	А			
SB Overall (Kenilworth Ave)	)	9.3	Α				11.1	в	1			9.3	Α				11.2	В			
Overall		15.1	В	962	Α	Pass		в	796	Α	Pass	15.1	В	965	Α	Pass	12.9	В	798	Α	Pass

Table 6-1:	Comparison of No-build and Build with Mitig	ation Condition Intersection AM and PM Peak Hou	r Operations Analysis (continued)

					No	-build	Condit	ion						Buil	d wit	h Mitig	ation C	ondi	ition		
			AM	Peak H	lour			PM	Peak H	our			AM	Peak H	lour			ΡM	Peak H	lour	
# Intersection and Approach	Lane Group	НСМ	2000	CL	V	Check	НСМ	2000	CLV		Check	HCM 2	2000	CLV	V	Check	HCM 2	000	CL\		Check
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Olleck	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Olieck	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Olleck	Delay (sec/ veh)		Critical Lane Vol	LOS	Check
10 Kenilworth Avenue (MD 201)	) & Ivy La	ane (Si	gnali	zed)			•							•		•					
EB (Ivy Ln)	R	0.1	Α				0.7	А				0.1	А				0.8	А			
EB Overall (Ivy Ln)		0.1	Α				0.7	Α				0.1	Α				0.8	Α			
NB (Kenilworth Ave)	L	18.6	В				25.8	С				18.6	В	1			25.8	С			
NB (Kenilworth Ave)	Т	0.3	Α				0.2	Α				0.3	А				0.2	А			
NB Overall (Kenilworth Ave)		3.4	Α				1.7	Α				3.4	Α				1.7	Α			
SB (Kenilworth Ave)	Т	0.7	Α				1.2	А				0.7	А				1.2	А			
SB (Kenilworth Ave)	R	0.0	Α				0.0	А				0.0	А				0.0	А			
SB Overall (Kenilworth Ave)		0.7	Α				1.2	Α				0.7	Α				1.2	Α			
Overall		2.3	Α	784	Α	Pass	1.3	Α	761	Α	Pass	2.3	Α	784	Α	Pass	1.3	Α	761	Α	Pass
11 Kenilworth Avenue/Edmons	ton Road	I (MD 2	201) &	Cherry	wood	d Lane	(Signa	alized	d)												
EB (Cherrywood Ln)	L	46.7	D				39.4	D				46.8	D				39.8	D			
EB (Cherrywood Ln)	R	40.7	D				33.8	С				40.7	D				33.3	С			
EB Overall (Cherrywood Ln)		45.7	D	1			37.5	D				45.8	D	1			37.6	D			
NB (Kenilworth Ave)	L	27.0	С	1			13.8	В				27.6	С	1			14.0	В			
NB (Kenilworth Ave)	Т	1.1	Α	1			1.2	А				1.1	А	1			1.3	А			
NB Overall (Kenilworth Ave)		11.1	В	1			3.5	Α				11.5	В	1			3.6	Α			
SB (Edmonston Rd)	Т	22.6	С	1			13.9	В				23.0	С	1			14.3	В			
SB (Edmonston Rd)	R	17.5	В	1			10.0	В				18.5	В	1			10.3	В			
SB Overall (Edmonston Rd)		21.2	С	1			13.2	В				21.7	С	1			13.6	В			
Overall		18.8	В	1,212	С	Pass	14.7	В	990	А	Pass	19.2	В	1,221	С	Pass	15.2	В	1,008	В	Pass
12 Edmonston Road (MD 201) &	Sunnys	ide Av	enue	(Signal	ized)																
EB (Sunnyside Ave)	L	108.9	F				113.0	F				36.3	D				57.3	Е			
EB (Sunnyside Ave)	R	66.9	Е				62.0	Е				23.6	С				32.5	С			
EB Overall (Sunnyside Ave)		77.9	E				80.1	F				27.0	С				41.2	D			
NB (Edmonston Rd)	L	102.8	F				98.0	F				19.1	В				27.8	С			
NB (Edmonston Rd)	Т	4.4	Α	]			18.3	В				3.8	А				8.3	А			
NB Overall (Edmonston Rd)		29.6	С				33.3	С				7.7	Α				11.9	В			
SB (Edmonston Rd)	Т	41.1	D				48.1	D				16.2	В				24.4	С			
SB (Edmonston Rd)	R	5.0	А				3.8	А				3.7	А				4.7	А			
SB Overall (Edmonston Rd)		35.6	D				41.4	D				14.3	В				21.4	С			
Overall		40.1	D	1,486	Е	Pass	46.7	D	1,692	F	Fail	13.8	В	1,015	В	Pass	21.7	С	1,188	С	Pass

Table o Transfer of the band and band with integration of an and the band of the band of the band and band with integration of the band and	Table 6-1:	Comparison of No-build and Build with Mitigation Condition Intersection AM and PM Peak Hour Operations Analysis (continued)
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					No	-build	Condi	tion						Buil	d wit	h Mitig	jation	Cond	ition		
			AM	Peak H	lour			PM	Peak l	Hour			AM	Peak H	lour			PM	Peak H	lour	
# Intersection and Approach	Lane Group	НСМ	2000	CL	V		НСМ	2000	CL	v		HCM :	2000	CL	V		НСМ	2000	CL	v	
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check
13 Edmonston Road (MD 201) &	Powder	· Mill R	oad (	Signali	zed)																
EB (Powder Mill Rd)	L	47.3	D				45.2	D				31.8	С				27.0	С			
EB (Powder Mill Rd)	Т	62.8	Е				81.1	F				38.0	D				47.5	D			
EB (Powder Mill Rd)	R	48.7	D				44.7	D				38.6	D				29.7	С			
EB Overall (Powder Mill Rd	)	52.8	D				60.5	Е				38.0	D				37.1	D			
WB (Powder Mill Rd)	L	57.0	Е				84.1	F				25.7	С				27.1	С			
WB (Powder Mill Rd)	Т	41.8	D				38.4	D				25.3	С	1			21.0	С			
WB (Powder Mill Rd)	R	35.6	D				34.1	С				21.6	С				18.6	В			
WB Overall (Powder Mill Ro	) (b	46.9	D				53.4	D				25.1	С				22.8	С			
NB (Edmonston Rd)	Ĺ	48.5	D				76.7	Е				18.7	В				29.5	С			
NB (Edmonston Rd)	Т	12.8	В				23.2	С				16.5	В				26.3	С			
NB (Edmonston Rd)	R	8.4	Α				12.5	В				10.5	В				11.2	В			
NB Overall (Edmonston Rd)		29.7	С				41.3	D				17.2	В				25.2	С	1		
SB (Edmonston Rd)	L	40.5	D				54.5	D				24.3	С				53.0	D			
SB (Edmonston Rd)	TR	52.5	D				60.4	Е				30.0	С				23.5	С			
SB Overall (Edmonston Rd)		52.0	D				59.8	Е				29.8	С				26.4	С			
Overall		42.5	D	1,593	Е	Pass	50.9	D	1,867	F	Fail	26.3	С	1,348	D	Pass	28.3	С	1,643	F	Fail
14 Greenbelt Metro Drive & Sit	e North A	Access	(Sigr	alized)	b																
EB (Greenbelt Metro Dr)	Т	N/A	N/A				N/A	N/A				3.3	А				11.0	В			
EB Overall (Greenbelt Metro	o Dr)	N/A	N/A				N/A	N/A				3.3	Α				11.0	В			
WB (Greenbelt Metro Dr)	L	N/A	N/A				N/A	N/A				-	-				-	-			
WB (Greenbelt Metro Dr)	Т	N/A	N/A				N/A	N/A				4.3	А				7.5	А			
WB Overall (Greenbelt Met	ro Dr)	N/A	N/A				N/A	N/A				4.3	Α				7.5	Α			
NB (Site North Access)	L	N/A	N/A				N/A	N/A				21.8	С				19.6	В			
NB (Site North Access)	R	N/A	N/A				N/A	N/A				21.3	С				16.8	В			
NB Overall (Site North Acce	ss)	N/A	N/A				N/A	N/A		1		21.7	С	<u> </u>			19.0	В	<u> </u>	,	
Overall		N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	4.7	Α	605	Α	Pass	12.9	В	1,029	В	Pass

						No	-build	Condit	ion						Buil	d wit	h Mitig	ation Cor	nditio	n	
				AM	Peak F	lour			РМ	Peak H	lour			AM	Peak H	lour		P	M Pe	ak Hou	•
#	Intersection and Approach	Lane Group	НСМ	2000	CL	v	Check	нсм	2000	CL		Check	HCM	2000	CLV	v	Check	HCM 200	D	CLV	Check
			Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	CHECK	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	CIIECK	Delay (sec/ LC veh)	S La	tical ane LO ol	
15	Greenbelt Station Bus Bays/	Greenbe	lt Met	ro Dri	ve & Gi	reenb	elt Sta	ation P	arkw	ay (Sig	nalize	ed)									
	EB (Greenbelt Sta Bus Bays)	LT	75.7	Е				54.0	D				76.9	Е				54.0 C	)		
	EB (Greenbelt Sta Bus Bays)	R	-	-				-	-				-	-							
	EB Overall (Greenbelt Sta B	us Bays)	75.7	Е				54.0	D				76.9	Ш				54.0 C	)		
	WB (Greenbelt Metro Dr)	L	56.6	Е				45.2	D				36.2	D				39.4 C	)		
	WB (Greenbelt Metro Dr)	Т	35.7	D				31.7	С				21.3	С				29.9 C	;		
	WB (Greenbelt Metro Dr)	R	36.0	D				31.5	С				21.7	С				36.8 C	)		
	WB Overall (Greenbelt Metro	o Dr)	52.1	D				41.6	D				32.6	С				37.4 C	)		
	NB (Greenbelt Sta Pkwy)	L	-	-				-	-				-	-							
	NB (Greenbelt Sta Pkwy)	Т	14.3	В				8.4	А				17.5	В				8.4 A	<b>`</b>		
	NB (Greenbelt Sta Pkwy)	R	13.8	В				21.4	С				22.5	С				13.0 E	5		
	NB Overall (Greenbelt Sta P	kwy)	14.0	В				16.2	В				19.8	В				11.3 E	5		
	Overall		31.4	С	644	Α	Pass	23.3	С	603	Α	Pass	26.6	С	682	Α	Pass	22.4 C	; 8	13 A	Pass
16	Greenbelt Station Parkway	& North C	Core D	evelo	pment/	Site I	Northw	/est Ac	cess	(Signal	ized)										-
	EB (North Core Dev)	L	69.2	Е				42.1	D				45.9	D				42.2 C	)		
	EB (North Core Dev)	TR	66.5	Е				35.0	С				43.2	D				35.1 C	)		
	EB Overall (North Core Dev)		68.8	Е				40.7	D				45.5	D				40.8 C	)		
	WB (Site Northwest Access)	LTR (AM)	-	-				-	-				47.0	D							
	WB (Site Northwest Access)	TR (PM)	-	-				-	-				-	-				48.2 C	)		
	WB (Site Northwest Access)	R (PM)	-	-				-	-				-	-				50.6 C	)		
	WB Overall (Site Northwest	Access)	-	-				-	-				47.0	D				49.4 C	)		
	NB (Greenbelt Sta Pkwy)	L	3.9	А				3.6	А				4.6	А				11.9 E	5		
	NB (Greenbelt Sta Pkwy)	TR	2.2	Α				3.4	А				2.6	А				11.0 E	5		
	NB Overall (Greenbelt Sta P	kwy)	2.7	Α				3.4	Α				2.9	Α				11.1 E	3		
	SB (Greenbelt Sta Pkwy)	TR	0.1	Α				0.1	Α				0.2	А				7.8 A			
	SB Overall (Greenbelt Sta P	kwy)	0.1	Α				0.1	Α				0.2	Α				7.8 A			
	Overall		5.4	Α	600	Α	Pass	11.0	В	460	Α	Pass	4.7	Α	976	Α	Pass	22.5 C	; 9	52 A	Pass
17	Greenbelt Station Parkway	& Reside	ntial A	cces	s to 500	Units	s (TWS	iC)													
	EB (Residential Access)	R	9.8	Α				9.3	А				10.0	А				9.3 A	1		
	EB Overall (Residential Acce	ess)	9.8	Α				9.3	Α				10.0	Α				9.3 A			
	Overall		0.6	-	N/A	N/A	Pass	0.2	-	N/A	N/A	Pass	0.5	-	N/A	N/A	Pass	0.2 -	N	/A N/.	A Pass

#### Table 6-1: Comparison of No-build and Build with Mitigation Condition Intersection AM and PM Peak Hour Operations Analysis (continued)

					No	-build	Condit	ion						Buil	d wit	h Mitig	jation	Cond	ition		
			AM	Peak H	lour			PM	Peak H	lour			AM	Peak H	lour	-		РМ	Peak H	lour	
# Intersection and Approach	Lane Group	HCM	2000	CL	v		НСМ	2000	CLV	/		HCM :	2000	CL۱	/		нсм	2000	CL	/	
		Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Chec
8 Greenbelt Station Parkway	& I-95/I-4	95 Off-	ramp	s/Site S	South	Acces	s/Kiss	& Rid	le (Sign	alize	d)										
EB (I-95 Off-ramps)	L	71.7	E				44.8	D				49.2	D				38.7	D			
EB (I-95 Off-ramps)	LTR	56.3	E				31.2	С				35.8	D				29.2	С			
EB Overall (I-95 Off-Ramps)	-	61.7	E				36.1	D				40.2	D				32.6	С			
EB (Kiss and Ride)	L	55.9	E				37.5	D				55.6	E				43.8	D			
EB Overall (Kiss and Ride)		55.9	E				37.5	D				55.6	E				43.8	D			
WB (Site South Access)	R	37.0	D				35.7	D				22.9	С				20.6	С			
WB Overall (Site South Acc	ess)	37.0	D				35.7	D				22.9	С	1			20.6	С			
NB (Greenbelt Sta Pkwy)	L	81.8	F				33.4	С				13.4	В	1			8.2	Α			
NB (Greenbelt Sta Pkwy)	Т	30.9	С				23.5	С				20.8	С				9.2	Α			
NB Overall (Greenbelt Sta F	y Pkwy)	32.7	С				24.3	С				20.6	С				9.2	Α			
SB (Greenbelt Sta Pkwy)	L	2.9	Α				84.5	F				53.4	D				1.0	Α			
SB (Greenbelt Sta Pkwy)	TR	6.6	А				76.4	Е				14.1	В				36.4	D			
SB Overall (Greenbelt Sta F	y Pkwy)	5.7	Α				77.7	Е				26.2	С				30.9	С			
Overall		40.0	D	950	Α	Pass	36.9	D	1,103	В	Pass	34.8	С	1,420	D	Pass	24.7	С	1,056	В	Pas
9 Greenbelt Station Parkway	& WMAT	A Gara	ige (S	Signaliz	.ed)		_				_										
EB (WMATA Garage)	L	76.3	Е				51.0	D				64.6	Е				51.0	D			
EB (WMATA Garage)	R	72.4	Е				37.8	D				49.1	D				37.8	D			
EB Overall (WMATA Garage	e)	74.9	E				49.3	D				59.0	Е				49.3	D			
NB (Greenbelt Sta Pkwy)	LT	65.7	Е				51.5	D				41.4	D	]			50.4	D			
NB (Greenbelt Sta Pkwy)	TR	3.0	А				4.6	Α				2.0	Α	1			4.6	Α			
NB Overall (Greenbelt Sta F	Pkwy)	34.7	С				28.3	С				21.7	С	1			27.7	С			
SB (Greenbelt Sta Pkwy)	Т	18.8	В				20.5	С				17.4	В	1			19.3	В			
SB (Greenbelt Sta Pkwy)	R	38.5	D				12.9	В				24.1	С	1			13.0	В			
SB Overall (Greenbelt Sta F	Pkwy)	25.5	С				20.3	С				19.7	В	1			19.2	В			
Overall		31.4	С	429	Α	Pass	27.8	С	524	Α	Pass	21.3	С	480	Α	Pass	27.1	С	524	Α	Pas

#### Comparison of No-build and Build with Mitigation Condition Intersection AM and PM Beak Hour Operations Analysis (continued) Table 6-1.

Table 6-1:	Comparison of No-build and Build with Mitigation Condition Intersection AM and PM Peak Hour Operations Analysis (continued)	

						No	-build	Condi	tion						Buil	d wit	h Mitig	jation	Cond	ition		
				AM	Peak H	lour		PM Peak Hour				AM	Peak H	lour			PM	Peak H	lour			
#	Intersection and Approach	Lane Group	НСМ	HCM 2000 CLV			НСМ	2000	CL			HCM 2	2000	CL	V		НСМ	2000	CL	v		
			Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check	Delay (sec/ veh)	LOS	Critical Lane Vol	LOS	Check
20	Greenbelt Station Parkway	& Reside	ntial A	cces	s to 300	) Unit	s (TWS	C)														
	EB (Residential Access)	LR	21.1	С				20.8	С				24.4	С				20.9	С			,
	EB Overall (Residential Acce	ess)	21.1	С				20.8	С				24.4	С				20.9	С			
	NB (Greenbelt Sta Pkwy)	LT	0.2	Α				0.8	Α	1			0.2	А	1			0.7	Α	1		
	NB Overall (Greenbelt Sta P	kwy)	0.1	-				0.3	-				0.1	-				0.3	-			
	Overall		1.5	-	N/A	N/A	Pass	0.6	-	N/A	N/A	Pass	1.6	-	N/A	N/A	Pass	0.6	-	N/A	N/A	Pass
21	Greenbelt Road (MD 193) & (	Greenbe	It Stati	on Pa	arkway	(Sigr	nalizec	l)														
	EB (Greenbelt Rd)	L	63.6	E				70.0	E				67.5	Е				70.0	E			
	EB (Greenbelt Rd)	Т	3.2	Α				8.0	А				3.3	А				8.0	Α			
	EB Overall (Greenbelt Rd)		11.5	В				12.6	В				14.0	В				12.7	В			
	WB (Greenbelt Rd)	Т	3.6	Α				4.9	Α				4.7	А				6.5	Α			
	WB (Greenbelt Rd)	R	0.1	Α				1.8	А				0.7	А				1.7	Α			
	WB Overall (Greenbelt Rd)		3.2	Α				4.5	Α				4.3	Α				5.8	Α			
	SB (Greenbelt Sta Pkwy)	L	67.1	E				59.9	E				68.2	Е	]			59.8	E			
	SB (Greenbelt Sta Pkwy)	R	46.0	D				47.4	D				48.3	D				47.3	D			
	SB Overall (Greenbelt Sta P	kwy)	57.5	E				54.1	D				59.2	E		•		54.0	D			
	Overall		11.1	В	988	Α	Pass	12.7	В	1,100	В	Pass	12.6	В	1,020	В	Pass	13.2	В	1,101	В	Pass

Notes:

AWSC = All-way STOP-Controlled intersection

EB = Eastbound, WB = Westbound, NB= Northbound, SB = Southbound

LTR = left / through / right lanes

LTR/LTR = No-build/Build with Mitigation

LOS = Level of Service

TWSC = Two-way STOP-Controlled unsignalized intersection (TWSC intersections do not have an overall LOS)

Delay is Measured in Seconds Per Vehicle.

Red cells denote intersections or approaches operating at unacceptable conditions.

^ Highway Capacity Manual was unable to report accurate delay using default gap acceptance values.

<sup>a</sup> Highway Capacity Software 2010 Roundabout results

<sup>b</sup> Signalized intersection would be part of the Build with Mitigation Condition, but was included as part of the No-build Condition provided by Renard Development Company, LLC.

### 6.6.5 Queuing Analysis

Synchro<sup>™</sup> was used to calculate the 50th percentile queue lengths, and SimTraffic<sup>™</sup> was used to calculate the 95th percentile queue lengths. The SimTraffic simulations have a statistical error of plus or minus 3.1 percent for the AM peak hour and 5.0 percent for the PM peak hour simulations. Based on the Synchro<sup>™</sup> and SimTraffic<sup>™</sup> analysis, there would be no signalized intersection approaches that would experience failing queue lengths in excess of 150 feet of the No-build Condition length.

### 6.6.5.1 Unsignalized Queuing Analysis

Based on the Synchro<sup>™</sup> and SimTraffic<sup>™</sup> analysis, there would be no unsignalized intersection approaches that would experience failing queue lengths in excess of 150 feet of the No-build Condition length.

### 6.6.5.2 Complete Intersection Queuing Analysis

The results of the No-build Condition compared with the Build with Mitigation Condition queuing analysis for both signalized and unsignalized intersections are presented in table 6-2. Note that the percentile values are expressed in feet, and a car occupies about 25 linear feet of roadway, including the space between cars.

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					No-build	Condition			Build with	Mitigation	)
			Turning	AM	Peak	PM F	Peak	AM F	Peak	PM F	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length (feet)	50th	95th	50th	95th	50th	95th	50th	95th
	Арргоаст	Group			Percentile						
			(1001)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
1	Greenbelt Road (MD 193) 8	& Cherryw	ood Lane	e/60th Ave	nue (Signa	alized)					
	EB (Greenbelt Rd)	L	350	132	165	240	250	130	164	221	255
	EB (Greenbelt Rd)	TR	1,584	148	128	373	294	134	126	427	379
	WB (Greenbelt Rd)	L	200	43	126	68	137	43	129	57	172
	WB (Greenbelt Rd)	TR	1,334	598	324	208	296	601	327	251	405
	NB (60th Ave)	L	318	-	-	-	-	55	102	54	101
	NB (60th Ave)	LTR/TR	318	132	217	154	#357	71	135	92	229
	SB (Cherrywood Ln)	L	350	74	112	172	254	75	107	194	271
	SB (Cherrywood Ln)	LT	1,300	75	134	178	315	76	132	198	399
	SB (Cherrywood Ln)	R	1,300	252	259	653	529	292	238	674	697
2	Cherrywood Lane & Breez	ewood D	rive (AWS	(C)							
	WB (Breezewood Dr)	LR	573	-	86	-	76	-	82	-	77
	NB (Cherrywood Ln)	Т	1,300	-	120	-	162	-	131	-	150
	NB (Cherrywood Ln)	R	1,300	-	81	-	113	-	84	-	107
	SB (Cherrywood Ln)	L	175	-	57	-	65	-	54	-	65
	SB (Cherrywood Ln)	Т	2,394	-	73	-	85	-	72	-	98
3	Cherrywood Lane & Spring	ghill Drive	e (TWSC)								
	WB (Springhill Dr)	LR	620	-	90	-	189	-	84	-	207
	NB (Cherrywood Ln)	TR	2,394	-	-	-	3	-	-	-	3
	SB (Cherrywood Ln)	L	350	-	53	-	68	-	52	-	67
4	Cherrywood Lane & Green	belt Metr	o Drive (R	Roundabo	ut)						
	EB (Greenbelt Metro Dr)	L	449	-	59	-	109	-	59	-	187
	EB (Greenbelt Metro Dr)	R	250	-	25	-	43	0	20	-	89
	NB (Cherrywood Ln)	LT	1,081	-	92	-	107	-	104	-	128
	SB (Cherrywood Ln)	Т	1,451	-	42	-	83	0	41	-	80
	SB (Cherrywood Ln)	R	200	-	13	-	10	-	20	-	7
5	Cherrywood Lane & Ivy La		C)		•		•		•	•	•
	EB (Cherrywood Ln)	LTR	1,451	-	156	-	45	-	159	-	50
	WB (Cherrywood Ln)	L	219	-	35	-	23	-	35	-	24
	WB (Cherrywood Ln)	TR	219	-	12	-	9	-	13	-	6
	NB (lvy Ln)	LT	485	-	81	-	131	-	82	-	146
	NB (lvy Ln)	R	485	-	38	-	53	-	37	-	54
	SB (Ivy Ln)	LTR	223	-	66	-	#287	-	69	-	#279

 Table 6-2:
 Comparison of No-build and Build with Mitigation Condition Queuing Analysis

	-				No-build	Condition			Build with	Mitigation	
			Turning	AM	Peak		Peak		Peak		Peak
#	Intersection and	Lane	Bay/Link	50th	95th	50th	95th	50th	95th	50th	95th
	Approach	Group	Length				Percentile				
			(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
6	Greenbelt Road (MD 193) 8	k 62 Aven	ue/Beltwa	y Plaza D	riveway (S	Signalized	)				
	EB (Greenbelt Rd)	L	250	0	27	9	63	1	25	12	96
	EB (Greenbelt Rd)	TR	1,334	63	56	511	221	64	57	676	398
	WB (Greenbelt Rd)	L	250	9	53	19	123	9	55	19	104
	WB (Greenbelt Rd)	Т	1,038	190	168	373	291	197	176	375	286
	WB (Greenbelt Rd)	R	1,038	0	39	3	96	0	34	3	99
	NB (62th Ave)	LTR	697	25	96	115	202	25	100	115	194
	SB (Beltway Plaza Drwy)	L	350	16	14	173	238	16	10	173	243
	SB (Beltway Plaza Drwy)	LT	472	17	69	172	268	17	67	172	277
	SB (Beltway Plaza Drwy)	R	350	0	23	0	51	0	22	0	73
7	Kenilworth Avenue (MD 20	1) & I-95/I	-495 SB O	ff-ramp (S	ignalized)		-				
	EB (I-95/I-495 SB Off-ramp)	L	531	112	300	97	211	112	288	97	202
	EB (I-95/I-495 SB Off-ramp)	R	736	0	394	0	2	0	316	0	-
	NB (Kenilworth Ave)	Т	1,263	46	90	66	116	47	91	66	118
	SB (Kenilworth Ave)	Т	574	229	180	56	115	229	171	57	119
8	Kenilworth Avenue (MD 20	1) & I-95/I	-495 NB O	ff-ramp (S	ignalized)						
	WB (I-95/I-495 NB Off-ramp)	L	885	223	245	160	222	223	246	160	226
	WB (I-95/I-495 NB Off-ramp)	R	835	217	152	61	96	217	153	61	88
	NB (Kenilworth Ave)	Т	345	116	131	49	94	119	140	49	96
	SB (Kenilworth Ave)	Т	199	56	154	77	129	56	154	78	134
9	Kenilworth Avenue (MD 20	-		/Marylanc			_				
	EB (Maryland SHA Office)	LTR	250	1	36	3	48	1	39	3	47
	WB (Crescent Rd)	LT	441	168	254	79	145	168	245	79	139
	WB (Crescent Rd)	R	250	0	133	0	71	0	129	0	69
	NB (Kenilworth Ave)	L	250	28	85	9	36	28	86	9	36
	NB (Kenilworth Ave)	Т	286	234	281	117	160	234	282	117	163
	NB (Kenilworth Ave)	R	250	9	114	2	35	9	119	2	41
	SB (Kenilworth Ave)	L	300	64	110	128	201	64	111	131	205
	SB (Kenilworth Ave)	Т	793	45	156	60	446	45	160	60	524
	SB (Kenilworth Ave)	R	793	0	10	0	194	0	10	0	224
10	Kenilworth Avenue (MD 20		ane (Sign	,							
	EB (Ivy Ln)	R	-	0	-	0	-	0	-	0	-
	NB (Kenilworth Ave)	L	547	88	134	21	59	87	139	21	58
	NB (Kenilworth Ave)	Т	-	45	64	29	-	45	73	29	-
	SB (Kenilworth Ave)	Т	1,198	4	93	15	101	4	85	16	102
	SB (Kenilworth Ave)	R	-	0	-	0	-	0	-	0	-

Table 6-2:	Comparison of No-build and Build with Mitigation Condition Queuing Analysis (continued)
	een parleen er ne sana and Bana marganen een ander gaeang / maryere (een ander)

	- 				No-build	Condition			Build with	Mitigation	
		• • • •	Turning	AMI	Peak	PM F	Peak	AM F	Peak	PM F	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length	50th	95th	50th	95th	50th	95th	50th	95th
	Арргоаст	Group	(feet)							Percentile	
			(1001)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)	(feet)
11	Kenilworth Avenue/Edmor	ston Roa	d (MD 201	) & Cherry	wood Lan	e (Signali	zed)				
	EB (Cherrywood Ln)	L	777	68	120	129	165	69	125	139	176
	EB (Cherrywood Ln)	R	1,304	0	65	0	200	0	61	0	198
	NB (Kenilworth Ave)	L	750	81	367	18	148	81	374	15	146
	NB (Kenilworth Ave)	Т	1,198	2	59	6	76	2	56	6	79
	SB (Edmonston Rd)	Т	594	307	301	212	204	311	281	217	192
	SB (Edmonston Rd)	R	250	31	#265	0	89	46	248	0	78
12	Edmonston Road (MD 201)	& Sunnys	side Aven	ue (Signal	ized)						
	EB (Sunnyside Ave)	L	953	182	555	320	#1234	71	127	231	658
	EB (Sunnyside Ave)	R	350	332	#421	455	#425	151	238	290	#434
	NB (Edmonston Rd)	L	450	362	387	268	#602	71	180	73	251
	NB (Edmonston Rd)	Т	964	249	259	809	#1865	70	110	222	199
	SB (Edmonston Rd)	Т	1,076	1336	#1629	1058	#1726	242	233	336	360
	SB (Edmonston Rd)	R	250	23	#293	14	#336	0	103	0	243
13	Edmonston Road (MD 201)	& Powde		d (Signali	· · · · · · · · · · · · · · · · · · ·						
	EB (Powder Mill Rd)	L	250	43	124	414	237	28	70	50	202
	EB (Powder Mill Rd)	Т	639	244	269	0	457	158	230	261	386
	EB (Powder Mill Rd)	R	500	0	83	0	154	67	110	72	101
	WB (Powder Mill Rd)	L	250	114	156	74	119	69	141	43	102
	WB (Powder Mill Rd)	Т	693	176	214	129	163	106	193	76	145
	WB (Powder Mill Rd)	R	100	0	100	0	62	0	90	0	43
	NB (Edmonston Rd)	L	541	513	364	~615	324	114	172	123	205
	NB (Edmonston Rd)	Т	641	274	246	19	297	246	274	445	458
	NB (Edmonston Rd)	R	325	0	20	64	96	0	35	0	271
	SB (Edmonston Rd)	L	275	21	104	0	140	12	63	39	107
	SB (Edmonston Rd)	TR	806	324	301	0	310	192	250	174	225
14	Greenbelt Metro Drive & S	ite North	Access (S	ignalized)	а						
	EB (Greenbelt Metro Dr)	Т	368	N/A	N/A	N/A	N/A	39	87	137	283
	WB (Greenbelt Metro Dr)	L	-	N/A	N/A	N/A	N/A	-	-	-	-
	WB (Greenbelt Metro Dr)	Т	237	N/A	N/A	N/A	N/A	78	131	61	138
	NB (Site North Access)	L	232	-	-	-	-	5	36	62	157
	NB (Site North Access)	R	232	N/A	N/A	N/A	N/A	0	33	0	107

#### Table 6-2: Comparison of No-build and Build with Mitigation Condition Queuing Analysis (continued)

	- 				No-build	Condition	-		Build with	Mitigation	า
			Turning	AM I	Peak	PM F	Peak	AM I	Peak	PM	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length (feet)	50th Percentile (feet)	(feet)	(feet)	(feet)	(feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)
15	Greenbelt Station Bus Bay		elt Metro	Drive & G	reenbelt S	tation Par	kway (Sig	nalized)			
	EB (Greenbelt Sta Bus Bays	LT	216	22	59	16	54	14	51	16	60
	EB (Greenbelt Sta Bus Bays	R	-	-	-	-	-	-	-	-	-
	WB (Greenbelt Metro Dr)	L	366	412	#446	169	250	278	362	163	250
	WB (Greenbelt Metro Dr)	Т	366	14	45	15	57	9	38	14	54
	WB (Greenbelt Metro Dr)	R	275	0	-	0	-	0	-	73	119
	NB (Greenbelt Sta Pkwy)	L	250	-	-	0	4	-	-	0	4
	NB (Greenbelt Sta Pkwy)	Т	243	100	102	50	84	69	-	61	88
	NB (Greenbelt Sta Pkwy)	R	243	31	-	12	11	22	112	22	31
16	Greenbelt Station Parkway	y & North	Core Dev	elopment/	Site North	west Acce	ss (Signal	ized)			
	EB (North Core Dev)	L	179	38	80	121	164	24	67	122	160
	EB (North Core Dev)	TR	179	0	36	0	63	0	35	0	114
	WB (Site Northwest Access)	LTR (AM)	-	-	-	-	-	0	48	-	-
	WB (Site Northwest Access)	TR (PM)	-	-	-	-	-	-	-	-	218
	WB (Site Northwest Access)	R (PM)	-	-	-	-	-	-	-	86	202
	NB (Greenbelt Sta Pkwy)	L	505	28	197	33	131	48	149	63	200
	NB (Greenbelt Sta Pkwy)	TR	505	28	107	67	228	65	143	136	305
	SB (Greenbelt Sta Pkwy)	TR	266	0	22	0	13	0	17	0	83
17	Greenbelt Station Parkway	y & Resid	ential Acc	ess to 500	Units (TW	SC)					•
	EB (Residential Access)	R	174	-	59	-	49	-	78	-	49
	SB (Greenbelt Sta Pkwy)	Т	-	-	-	-	-	-	5	-	-
	NB (Greenbelt Sta Pkwy)	Т	459	-	3	-	302	-	-	-	65
18	Greenbelt Station Parkway	y & I-95/I-4	495 Off-rai	nps/Site S	South Acce	ss/Kiss & I	Ride (Sign	alized)	ł		
	EB (I-95 Off-ramps)	L	188	238	223	187	134	360	422	195	149
	EB (I-95 Off-ramps)	LTR	188	129	222	21	153	321	470	30	132
	EB (Kiss and Ride)	L	160	229	#258	116	174	85	#217	62	110
	WB (Site South Access)	R	402	6	27	118	160	4	25	90	154
	NB (Greenbelt Sta Pkwy)	L	375	24	59	35	76	10	47	7	56
	NB (Greenbelt Sta Pkwy)	Т	530	325	86	110	87	220	88	58	93
	SB (Greenbelt Sta Pkwy)	L	400	0	120	0	54	~120	345	0	38
	SB (Greenbelt Sta Pkwy)	TR	459	0	73	28	93	0	192	15	69

 Table 6-2:
 Comparison of No-build and Build with Mitigation Condition Queuing Analysis (continued)

					No-build	Condition			Build with	Mitigatior	h
	Interception and	Long	Turning	AM	Peak	PM F	Peak	AM F	Peak	PM	Peak
#	Intersection and Approach	Lane Group	Bay/Link Length (feet)	50th	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)	50th Percentile (feet)	95th Percentile (feet)
19	Greenbelt Station Parkway	y & WMA	TA Garage	) (Signaliz	:ed)						
	EB (WMATA Garage)	L	150	7	30	100	#158	4	28	100	#162
!	EB (WMATA Garage)	R	290	0	24	0	63	0	22	0	83
	NB (Greenbelt Sta Pkwy)	LT	330	358	183	157	80	237	142	158	87
'	NB (Greenbelt Sta Pkwy)	TR	330	4	145	48	99	0	109	48	90
	SB (Greenbelt Sta Pkwy)	Т	162	141	68	248	152	112	55	220	#177
'	SB (Greenbelt Sta Pkwy)	R	162	23	14	0	2	15	13	0	2
20	Greenbelt Station Parkway	y & Resid	ential Acc	ess to 300	Units (TW	SC)			<u> </u>		
	EB (Residential Access)	LR	224	-	64	-	44	-	60	-	45
'	NB (Greenbelt Sta Pkwy)	LT	345	_	0	-	0	-	0	-	0
	SB (Greenbelt Sta Pkwy)	TR	350	-	5	-	6	-	2	-	8
21	Greenbelt Road (MD 193) 8	Greenb	elt Station	Parkway	(Signalize	d)				-	
	EB (Greenbelt Rd)	L	57	95	144	97	#142	124	169	98	#151
	EB (Greenbelt Rd)	Т	1,008	84	95	360	233	84	106	360	242
	WB (Greenbelt Rd)	Т	1,584	117	130	165	199	114	121	200	196
'	WB (Greenbelt Rd)	R	150	0	71	19	#167	11	52	18	#178
	SB (Greenbelt Sta Pkwy)	L	524	115	162	125	185	109	143	125	198
	SB (Greenbelt Sta Pkwy)	R	225	165	209	184	#242	145	204	185	#238

 Table 6-2:
 Comparison of No-build and Build with Mitigation Condition Queuing Analysis (continued)

Notes:

~ 50th percentile volume exceeds capacity, queue is theoretically infinite.

# 95th percentile volume exceeds capacity, queue may be longer.

m Volume for 95th percentile queue is metered by upstream signal. Due to upstream metering, the 95th percentile queue may be less than the 50th percentile queue.

AWSC = All-way STOP-Controlled intersection

EB = Eastbound, WB = Westbound, NB= Northbound, SB = Southbound

LTR = left / through / right lanes

TWSC = Two-way STOP-Controlled intersection

Red cells denote approaches and lane groups whose queuing length exceeds capacity.

<sup>a</sup> Signalized intersection would be part of the Build with Mitigation Condition, but was included as part of the No-build Condition provided by Renard Development Company, LLC.

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## 6.6.6 Recommend Traffic Mitigation

Recommended traffic mitigation measures were developed to address the substantial traffic impacts caused by the addition of the Consolidated FBI HQ in Greenbelt. These included traffic signal optimization, road widening, lane geometry improvements at intersections, installation of new traffic signals, lane striping adjustments. If implemented, the recommended traffic mitigation measures would maintain acceptable traffic flow conditions based on the Greenbelt Site Transportation Agreement. The following recommendations in table 6-3 are provided to mitigate the proposed traffic impacts of the Greenbelt Build Condition:

Table 6-3: Recommended Traffic Mitiga	
Impact	Mitigation
To improve traffic operations along Greenbelt Station Parkway and Greenbelt Road the traffic signals would be optimized and/or coordinated	<ul> <li>Optimize the traffic signals at the following locations:</li> <li>Greenbelt Road (MD 193) and Greenbelt Station Parkway intersection</li> <li>Greenbelt Station Parkway and WMATA Garage intersection</li> <li>Greenbelt Station Parkway and North Core Mixed Use/Site Northwest Access intersection</li> <li>Greenbelt Station Parkway and Greenbelt Metro Drive intersection</li> </ul>
To improve traffic operations along the Edmonston Road corridor widen the road, change the intersection geometry including new turn lanes (optimize traffic signal if warranted)	<ul> <li>Widen the road along Edmonston Road between Powder Mill Road and 1,500 feet south of Sunnyside Road and change the lane geometry at the following locations: <ul> <li>Edmonston Road (MD 201) and Powder Mill Road intersection</li> <li>Edmonston Road (MD 201) and Sunnyside Road intersection</li> </ul> </li> </ul>
To improve traffic operations at isolated locations change the intersection geometry and optimize traffic signal if warranted	<ul> <li>Change the intersection geometry at the following locations:</li> <li>Greenbelt Road (MD 193) and Cherrywood Lane/60th Avenue intersection</li> <li>Greenbelt Station Parkway and I-95/I-495 off-ramp/Site South Access intersection</li> </ul>
To improve traffic operations at isolated locations install new traffic signals	Install a new traffic signal at Greenbelt Metro Drive and Site North Access intersection
To improve traffic operations along ramp connecting the Interstate to the planned WMATA garage and Greenbelt Station Parkway	Revise the lane striping plane along I-95/I-495 off- ramp from the Interstates to Greenbelt Station Parkway to provide one lane that leads directly to the WMATA garage

Table 6-3:	<b>Recommended Traffic Mitigation</b>
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The mitigation measures were developed to ensure the intersections would operate in a safe manner for all modes. This included assigning adequate pedestrian crossing times for any signalized intersection that required a change in the number of approach lanes and recommending non-motorized bridges to ensure bicycle and pedestrians can safely cross when an at grade crossing would not be safely accommodated. It is assumed that all planned roadway improvements and mitigation would follow the American Association of State Highway

Transportation Officials, Maryland SHA, M-NCPPC, and Prince George's County requirements to ensure all vehicle, bicycle, and pedestrian movements are designed to the latest safety standards.

Overall, the study area would experience isolated intersection improvements, specifically along Edmonston Road. These improvements would result in changing the impacts from direct, long-term, adverse impacts to direct, long-term, beneficial impacts because the operations would improve to a better operation than the No-build Condition. In addition to these impacts, there would be two failing Interstate facilities: one would be caused by the volume of vehicles added to the I-95/I-495 northbound off-ramp to Landover Road during the AM peak hour, and the second would be caused by the volume of vehicles added to the I-95/I-495 southbound on-ramp from Arena Drive during the PM peak hour. These area-wide impacts would result in direct, long-term, major adverse impacts due to the regional nature of the Interstate system (see Section 6.6.7.3 for further information).

The construction impacts would change from direct, short-term, adverse impacts under the Build Condition to direct, short-term, major adverse impacts under the Build with Mitigation Condition during the construction period. This change in impact level reflects the short-term impacts from adding construction-related trips caused by trucks, employees, and equipment as well as intermittent lane or road closures at the Greenbelt site and locations where the roadway improvements would occur.

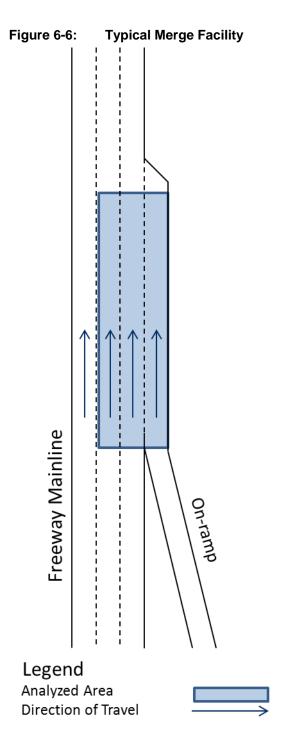
## 6.6.7 Freeway Analysis Summary

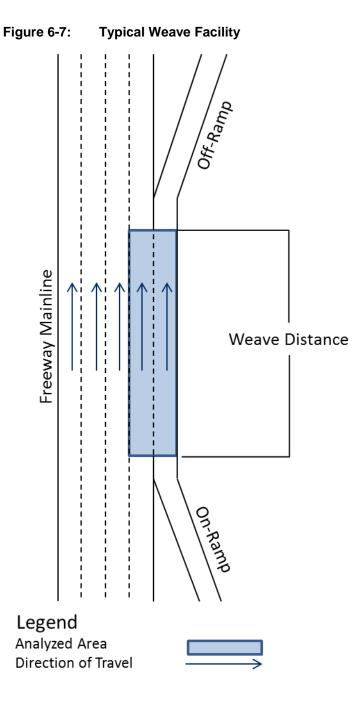
The *Highway Capacity Software* (HCS) Version 6.65 was used to determine the Interstate operations for these key on- and off-ramps. The HCS modules follow the HCM uninterrupted flow procedures called freeways. The Interstate system is a network of signed roadways that crisscross the country from coast to coast (east-west) and border to border (north-south) and operate as freeways or uninterrupted vehicle flow. Interrupted vehicle flow refers to the roadways with traffic signals, stop signs, and roundabouts. Based on the proposed FBI trip distribution, 86 percent of forecasted FBI vehicle trips would use the Interstate system (I-95/I-495) to access the proposed site. Because the interstate system is vital to serving the Greenbelt site, the Interstates were evaluated to determine whether or not the added vehicle trips would cause any failing interstate facilities.

Based on the agreed Greenbelt Site Transportation Agreement (Appendix C1), the evaluated Interstate facilities focused on the peak direction only and at the primary off-ramps serving the inbound forecasted FBI vehicle trips during the AM peak hour and the on-ramps serving the outbound forecasted FBI vehicle trips during the PM peak hour.

### 6.6.7.1 Freeway Facilities Types Studied

Several freeway facility types were evaluated, including merge and weave designs. In total, the analysis included the evaluation of one merge and four weave facilities. Merge facilities represent an on-ramp to the freeway. Weave facilities represent an on-ramp followed by an off-ramp that share the same lane and are spaced close enough to create a crisscross vehicle pattern caused by vehicles entering the freeway, potentially blocking vehicles exiting the freeway or vice versa. The vehicle volumes combined with the distance between the on- and off-ramps help determine whether or not a facility qualifies as a weave or two separate merge and diverge areas (HCM, Equation 12-4; TRB 2010). Figure 6-6 illustrates a typical merge facility, and figure 6-7 illustrates a typical weave facility.





Freeway facilities are evaluated based on the density of vehicles. The higher the density, the slower the vehicles travel, and the worse the operations. Based on the vehicle density, the HCM provides LOS equivalents to represent the driver's perception of the facility operation. Table 6-4 contains the HCM freeway LOS.

#### Table 6-4: HCM Weaving Segments, Merge, and Diverge Facilities Level of Service

LOS	Density (passenger cars/mile/lane)	Description
А	Less than or equal to 10	
В	>10-20	Passing operation
С	>20-28	
D	>28-35	
E	>35	Unstable conditions
F	Demand Exceeds Capacity	Above capacity and unstable conditions

Source: TRB (2010)

All Interstate facilities were evaluated based on a PHF of 0.92 (ratio of the 60-minute volume divided by 4 times the highest 15-minute volume), the lowest accepted by VDOT's *Traffic Impact Analysis Regulations* to be consistent for all three sites and provide a conservative value for the analysis of future facilities (VDOT 2012). This is also the same PHF used to evaluate all intersection facilities within the study area.

#### 6.6.7.2 Freeway Facilities Evaluated

The following facilities were evaluated:

AM Peak Hour Inbound Flows

- Weave Section: I-95/I-495 northbound between Kenilworth Avenue (MD 201) and Greenbelt Station Parkway/Greenbelt Metro Station
- Weave Section: I-95/I-495 southbound between U.S. Route 1 and Greenbelt Station Parkway/Greenbelt Metro Station

PM Peak Hour Inbound Flows

- Weave Section: I-95/I-495 northbound between Greenbelt Station Parkway and U.S. Route 1
- Weave Section: I-95/I-495 southbound between Greenbelt Station Parkway and Kenilworth Avenue (MD 201)
- Ramp Merge: I-95/I-495 northbound from Greenbelt Station Parkway/Greenbelt Metro Station (Nobuild comparison due to failing Build Condition)

#### I-95/I-495 Northbound between Kenilworth Avenue (MD 201) and Greenbelt Station Parkway/Greenbelt Metro Station

This facility is a five-lane facility along the freeway mainline with four through lanes and one lane serving the onand -off ramps. There is a 2,785-foot distance between the on- and off- ramps and two maneuvering lanes (minimum number of lanes in use to either enter or exit the freeway).

#### I-95/495 Southbound between U.S. Route 1 and Greenbelt Station Parkway/Greenbelt Metro Station

This facility is a five-lane facility along the freeway mainline with four through lanes and one lane serving the onand off-ramps. There is a 3,500-foot distance between the on- and off-ramps and two maneuvering lanes (minimum number of lanes in use to either enter or exit the freeway).

#### I-95/495 Northbound between Greenbelt Station Parkway/Greenbelt Metro Station and U.S. Route 1

This facility is a five-lane facility along the freeway mainline with four through lanes and one lane serving the onand -off ramps. There is a 4,500-foot distance between the on- and off- ramps and two maneuvering lanes (minimum number of lanes in use to either enter or exit the freeway).

# I-95/I-495 Southbound between Greenbelt Station Parkway/Greenbelt Metro Station and Kenilworth Avenue (MD 201)

This facility is a five-lane facility along the freeway mainline with four through lanes and one lane serving the onand off-ramps. There is a 3,500-foot distance between the on- and off-ramps and two maneuvering lanes (minimum number of lanes in use to either enter or exit the freeway).

# I-95/I-495 Northbound from Greenbelt Station Parkway/Greenbelt Metro Station (No-build Condition only as comparison to failing Build Condition weave facility)

This facility is a five-lane facility with four through lanes and one lane serving the on-ramp. There is a 1,000-foot deceleration lane serving the on-ramp. Based on the HCM (equation 12-4; TRB 2010) the vehicle volume entering, exiting, and remaining on the freeway determines the maximum distance for a facility to be considered a weave facility. Because the vehicle volume between Greenbelt Station Parkway/Greenbelt Metro Station and the downstream off-ramp to U.S. Route 1 result in weave distance shorter than the actual distance, this facility does not qualify to be analyzed as a weave facility and must be analyzed as a merge facility.

#### 6.6.7.3 Freeway Analysis

Based on the analysis performed using HCS, two Interstate facilities are projected to fail. During the AM peak hour, the weave facility serving FBI vehicle trips from I-95 from the north to Greenbelt Station Parkway/Greenbelt Metro Station would result in a failing freeway facility (LOS F). During the PM peak hour, the weave facility serving FBI vehicle trips to I-95 to the north would result in a failing freeway facility (LOS F). Table 6-5 contains the Build with Mitigation Condition HCS freeway analysis.

Freeway Analysis	Facility Type	Density (pc/mi/ln)	LOS	Check
I-95/I-495 Northbound between Kenilworth Avenue (MD 201) and Greenbelt Station Parkway/Greenbelt Metro Station (AM only)	Weave	30.7	D	Pass
I-95/I-495 Southbound between U.S. Route 1 and Greenbelt Station Parkway/Greenbelt Metro Station_(AM only)	Weave	44.5	F	Fail
I-95/I-495 Northbound between Greenbelt Station Parkway/Greenbelt Metro Station and U.S. Route 1 (PM only)	Weave	38.0	Е	Fail
I-95/I-495 Southbound between Greenbelt Station Parkway/Greenbelt Metro Station and Kenilworth Avenue (MD 201) (PM only)	Weave	32.8	D	Pass

#### Table 6-5: Build with Mitigation Condition Freeway Analysis

Notes: LOS = Level of Service; Density = Passenger cars per mile per lane (pc/mi/ln)

If any of the Interstate facilities failed, an additional test was agreed based on the Greenbelt Site Transportation Agreement to determine if the difference in vehicle density between the No-build Condition and Build Condition was greater than 5 percent. This would confirm that the forecasted FBI vehicle trips significantly contributed to the failing of the facilities. Based on the additional analysis, the failing Interstate facilities would contribute more than 5 percent to vehicle density, thus both facilities would be impacted by the addition of forecasted FBI vehicle trips. It should be noted that this analysis followed the Maryland SHA future planned designs for the I-95/I-495 corridor between U.S. Route 1 and Kenilworth Avenue (MD 201). Based on a conversation with Maryland SHA, the Maryland SHA analysis performed indicated that the facilities with failing LOS would be expected to fail in the future; however, the average speed through the corridor would be expected to be 30 mph and thus acceptable (Maryland SHA 2015b). Table 6-6 contains the Build with Mitigation Condition additional freeway analysis.

Additional Freeway Analysis	Condition	Density (pc/mi/ln)	Density Difference	AM Check
I-95/I-495 Southbound between U.S. Route 1 and Greenbelt	No-build	39.7		
Station Parkway/Greenbelt Metro Station_(AM only)	Build with Mitigation	44.5	12.1%	Fail
I-95/I-495 Northbound between Greenbelt Station	No-build	29.0 ª		
Parkway/Greenbelt Metro Station and U.S. Route 1 (PM only)	Build with Mitigation	38.0	31.0%	Fail

#### Table 6-6: Build with Mitigation Condition Freeway Analysis

<sup>a</sup> Represents a Merge Facility

Notes: Density = Passenger cars per mile per lane (pc/mi/ln)

## 6.6.8 Entry Control Facility Summary

The ECF analysis was performed once the complete set of external roadway mitigation was established. All mitigation measures were coded into TransModeler<sup>™</sup>, and the several scenarios were tested to determine the minimum number of lanes capable of handling the AM peak hour forecasted FBI vehicle trips. It was determined that three lanes at the Site South Access and three lanes at the Site Northwest Access were required to handle the forecasted demand. This resulted in the following breakdown of vehicles between the two ECFs:

•	South Entrance from Frontier Drive Extension:	491 vehicles	or	48 percent
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East Entrance from Metropolitan Center Drive: 530 vehicles or 52 percent

Following the process to ensure statistical accuracy for the simulations, TransModeler<sup>™</sup> was used to run 25 simulations for each scenario to calculate the standard deviation based on the VHT metric. Appendix C10 contains the statistical results for determining the minimum number of TransModeler<sup>™</sup> simulations required to be within plus or minus 2 percent at the 95th percentile confidence interval. Following the statistical procedure, the following three scenarios were completed:

- Site South Access and Greenbelt Station Parkway and Site Northwest Access and Greenbelt Station Parkway traffic signal controlled
  - 1. Two lanes at the Site South Access and two lanes at the Site Northwest Access
  - 2. Three lanes at the Site South Access and two lanes at the Site Northwest Access
  - 3. Three lanes at the Site South Access and three lanes at the Site Northwest Access

The first and third scenarios relied on the inbound FBI vehicle trip volume from each origin to the two ECFs calculated using the TransModeler<sup>™</sup> DTA process. Because the distance between the two ECFs to the decision point is too short for vehicles to decide whether to use the Site South Access or Site Northwest Access, the DTA was unable to properly balance the FBI vehicle volumes between the two ECFs. Therefore, the second scenario required a manual adjustment to the inbound FBI vehicle trip volume to balance the volumes between the two

ECFs based on an imbalance in the number of lanes (three in the Site South Access and two in the Site Northwest Access). A total of 50 vehicles (30 from I-95/I-495 North and 20 from I-95/I-495 South) were shifted from the Site Northwest Access to the Site South Access to account for the higher capacity available at the Site South Access.

Based on the analysis, a minimum of three lanes for both ECFs would be required for the average queue length for all lanes exceeding the average available space for all lanes. The second scenario was close, but the average queue length for all lanes still exceeded the average capacity by 17 feet for the Site South Access and 12 feet for the Site Northwest Access. Two lanes for both ECFs resulted in substantial queues for both facilities. Table 6-7 contains the ECF results.

#### Table 6-7: ECF Results

		Two and Two Lanes					Three and Two Lanes				Three and Three Lanes					
Entrance		Vehicles Processed	Proposed Length	Average Queue	Maximum Queue		Vehicles Processed	Proposed Length	Average Queue	Maximum Queue		Vehicles Processed	Proposed Length	Average Queue	Maximum Queue	
	Lanes	Vehicles		Feet		Pass/Fail	Vehicles		Feet		Pass/Fail	Vehicles		Feet		Pass/Fail
	1	218	295	285	1,155	Fail	203	295	129	388	Fail	152	295	64	208	Pass
Site South	2	217	305	352	2,043	Fail	211	305	131	291	Pass	187	305	74	181	Pass
Access	3						211	315	132	272	Pass	194	315	81	193	Pass
	A	verage	300		1,599	Fail		300		317	Fail		300		194	Pass
	1	209	480	334	754	Fail	211	480	231	575	Fail	157	480	69	199	Pass
Site Northwest	2	217	495	361	849	Fail	212	495	213	423	Pass	175	170	77	207	Pass
Access	3											200	495	118	266	Pass
	A	verage	488		802	Fail		488		499	Fail		382		224	

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### 6.6.9 Signal Warrant Analysis Summary

A signal warrant analysis is a quantitative assessment based on traffic volumes and established standards to determine whether or not installing a traffic signal at a specific intersection is justified or warranted. A signal warrant analysis was conducted following the guidelines from the *2009 Manual on Uniform Traffic Control Devices* (MUTCD) (FHWA 2012). To be consistent for all three proposed alternative sites, the Virginia Supplement to the 2009 MUTCD, 2011 Edition guidelines were also employed (VDOT 2011). Combining both methods provides an analysis of two signal warrants per intersection: an average daily traffic (ADT) warrant and a peak hour warrant.

The ADT warrant (following the Virginia guidelines) compares a forecasted ADT volume for the intersection to minimum established ADTs based on the number of lanes along the two intersecting roadways. The forecasted intersection ADT is calculated by applying a 10 percent factor to the AM peak hour forecasted volumes (highest left-turn volume). The volumes are then compared to several tables in the VDOT MUTCD Supplement. The first table in the VDOT MUTCD Supplement contains the urban area minimum vehicle volumes to qualify the intersection; the second table in the VDOT MUTCD Supplement contains the urban area interruption of continuous traffic vehicle volumes to qualify the intersection. Both tables also contain 80th percentile volumes for both cases, which is used in urban areas. Based on the ADT warrant analysis, the Greenbelt Metro Drive and Site North Access that could benefit from a traffic signal would not meet all the ADT warrants. Table 6-8 contains the ADT warrant summary.

Warrant	Forecasted ADT	Warrant Minimum Limit	Warrant Check
Greenbelt Metro Drive and Site	North Access		
Warrant 1A – Minimum Vehicular Volume	14,980	8,000	Meets
Warrant 1B – Interruption of Continuous Traffic	14,980	12,000	Meets
Marrant 1C Combination of 1A and 1D (80%)	14,980	6,400	Meets
Warrant 1C – Combination of 1A and 1B (80%)	14,980	9,600	Meets

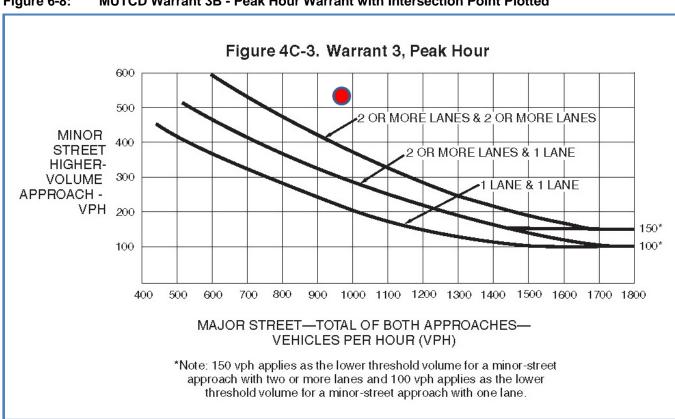
#### Table 6-8:ADT Warrant Analysis

The peak hour warrant following the MUTCD requires two categorical tests. If either of the categorical tests passes, then the intersection meets the warrant. The first category includes three tests: a test of the intersection delay under STOP-sign control, a test of the minor street vehicle volume, and a test of the total intersection volume. The intersection delay test determines if the intersection is under a STOP-control, the delay for the minor-street would exceed five vehicle-hours (number of vehicles in queue times approach vehicle delay) for two lanes. The minor street vehicle volume test determines whether or not the vehicle volume exceeds 150 vehicles for two lanes. The third test of the total intersection volume examines if the total volume entering the intersection exceeds 650 vehicles for a three lane approach. The second categorical test includes one test based on a plotted chart published in the MUTCD (figure 4C-3; FHWA 2012). The chart plots the highest minor street approach volume against the total major street approach volumes. If the plotted point for the highest minor street approach falls higher than the appropriate curve (based on number of lanes for the major and minor approaches), the peak hour warrant is met.

Based on the peak hour warrant analysis, the intersection would meet the warrant. The intersection meets all parts of the peak hour warrant except for the total stopping time; however, as long as the intersection meets the second category, the warrant is met. Table 6-9 contains the peak hour warrant analysis results. Figure 6-8 shows the MUTCD plotted graph with the intersection point plotted.

#### Table 6-9: Peak Hour Warrant Analysis

Warrant	Forecasted Values	Warrant Minimum Limit	Category Check	Overall Check
Greenbelt Metro	Drive and Site No	orth Access		
Warrant 3A1 – Total Stopping Time	2.5 hours	4 hours	Fails	
Warrant 3A2 – Minor Street Volume	522 vehicles	150 vehicles	Meets	
Warrant 3A3 – Total Entering Volume	1,498 vehicles	650 vehicles	Meets	
Warrant 3B – Plotted Point on Curve	See fi	gure 6-8	Meets	Meets



#### Figure 6-8: MUTCD Warrant 3B - Peak Hour Warrant with Intersection Point Plotted

### 6.7 Overall Summary

The following summarizes the conclusions of the transportation evaluation:

A total of 3,296 AM peak hour and 3,047 PM peak hour person trips are projected to be added to all modes of transportation. Total Metro transit trips are projected to be 1,742 trips in the AM peak hour and 1,610 trips in the PM peak hour. Total vehicle trips are projected to be 1,100 trips in the AM peak hour and 1,016 trips in the PM peak hour. The remaining trips would be commuter rail, bicycle, or walking trips.

The pedestrian network would expand under the No-build Condition with the inclusion of Greenbelt Station Parkway providing a new connection between the Greenbelt Metro Station and Greenbelt Road serving North and South Core developments. The inclusion of the Greenbelt site would allow for the same connections as the Nobuild Condition. It is assumed that all sidewalk curb ramps located adjacent to the parcel would be constructed to ADA compliance.

The bicycle network would expand with the inclusion of Greenbelt Station Parkway providing a new connection between the Greenbelt Metro Station and Greenbelt Road serving the North and South Core developments. The inclusion of the Greenbelt site would not change the bicycle connections. These new connections would provide for an interconnected bicycle network linking all proposed bicycle facilities in the study area and would encourage bicycle use to access to the Greenbelt site.

The transit network (Metrorail and Metrobus) would not be affected by the Greenbelt Site. The Greenbelt Metro Station and all bus service would operate below capacity with the addition of the forecasted background growth and transit trips. It is assumed that WMATA would follow their long-term plan to address growth-related capacity issues for both bus and rail operations.

Parking availability would remain the same because the Greenbelt site would accommodate all parking needs onsite and implement a robust Transportation Management Plan to discourage employees from seeking alternative parking options in the nearby neighborhoods.

Truck access would be designed to accommodate the Greenbelt site from the Greenbelt Station Parkway site south access. This plan is not the official plan, but a plan to evaluate as part of the EIS. The Greenbelt Station Parkway site south access would operate as a truck only access point during off-peak hours because it would be assumed that all truck deliveries would be scheduled during the off-peak hours.

The traffic operations at two intersections (Edmonson Road at Powder Mill Road and Kenilworth Avenue at I-95/I-495 Southbound off-ramp) currently operates at an unacceptable level of service under the Existing Condition. Once the background growth, planned developments, and planned improvements are added (No-build Condition), the same intersection would continue to fail. There are a number of planned roadway improvements within the Springfield site study area to compensate for the vehicle trips added from the background growth.

The addition of the Greenbelt site to the traffic network would result in three intersections operating at an unacceptable level of service. These three failing intersections would experience equal or better operations than the No-build Condition as a result of recommended mitigation that include new turning lanes, extended turning lane lengths, and new travel lanes. Overall, the roadway non-Interstate network would operate much better and experience shorter queues with the addition of the recommended mitigation when compared to the No-build Condition.

There are forecasted to be two failing Interstate facilities that directly serve access between the Capital Beltway and the Greenbelt site. The Maryland SHA is working to determine the best course of action to address these issues. It is assumed, at a minimum, there will be required changes to the Interstate ramps along the Capital Beltway between the U.S. Route 1 and Baltimore Washington Memorial Parkway Interchanges.

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## 8.0 Acronyms and Abbreviations

Α	
AADT	Annual average daily traffic
ADA	Americans with Disabilities Act
ADT	Average daily traffic
ATR	Automated Traffic Recorder
AWSC	All-way STOP-Controlled
С	
CEQ	Council on Environmental Quality
CFR	Code of Federal Regulations
CLV	Critical Lane Volume
CMRT	Central Maryland Regional Transit
CUP	Central Utility Plant
D	
DDOT	District Department of Transportation
DOT	Department of Transportation
DTA	dynamic traffic assignment
E	
ECF	Entry Control Facility
EIS	Environmental Impact Statement
F	
FBI	Federal Bureau of Investigation
FHWA	Federal Highway Administration
G	
GIS	Geographic Information Systems
GSA	General Services Administration
GSF	Gross Square Feet

н	
НСМ	Highway Capacity Manual
HCS	Highway Capacity Software
HQ	Headquarters
I	
ISC	Interagency Security Committee
ITE	Institute of Transportation Engineers
J	
JEH	J. Edgar Hoover
L	
LOS	Level of Service
М	
MARC	Maryland Area Regional Commuter
MEV	million entering vehicles
M-NCPPC	Maryland National Capital Park and Planning Commission
mph	miles per hour
Maryland SHA	State Highway Administration
MTA	Maryland Transit Administration
MUTCD	Manual on Uniform Traffic Control Devices
MWCOG	Metropolitan Washington Council of Governments
Ν	
NCHRP	National Cooperative Highway Research Program
NCPC	National Capital Planning Commission
NCR	National Capital Region
NEPA	National Environmental Policy Act
NFPA	National Fire Protection Association

0	
OPO	Old Post Office
Р	
PHF	peak hour factor
R	
RDF	Remote Delivery Facility
RFDS	Reasonably Foreseeable Development Scenario
S	
SDDCTEA	Surface Deployment and Distribution Command Transportation Engineering Agency
SF	Square Foot
SMA	Sectional Map Amendment
SOV	Single Occupant Vehicle
т	
TAZ	Transportation Analysis Zone
TDM	Travel Demand Management
ΤΙΑ	Transportation Impact Assessment
TIP	Transportation Improvement Program
TMP	Transportation Management Plan
TRB	Transportation Research Board
TWSC	Two-way STOP-Controlled
U	
UMD	University of Maryland
U.S.	United States
USDA	U.S. Department of Agriculture
USDOJ	U.S. Department of Justice

V	
v/c	volume-to-capacity ratio
VC	Visitor Center
VDOT	Virginia Department of Transportation
VHT	Vehicle hours of travel
W	
WMATA	Washington Metropolitan Area Transit Authority